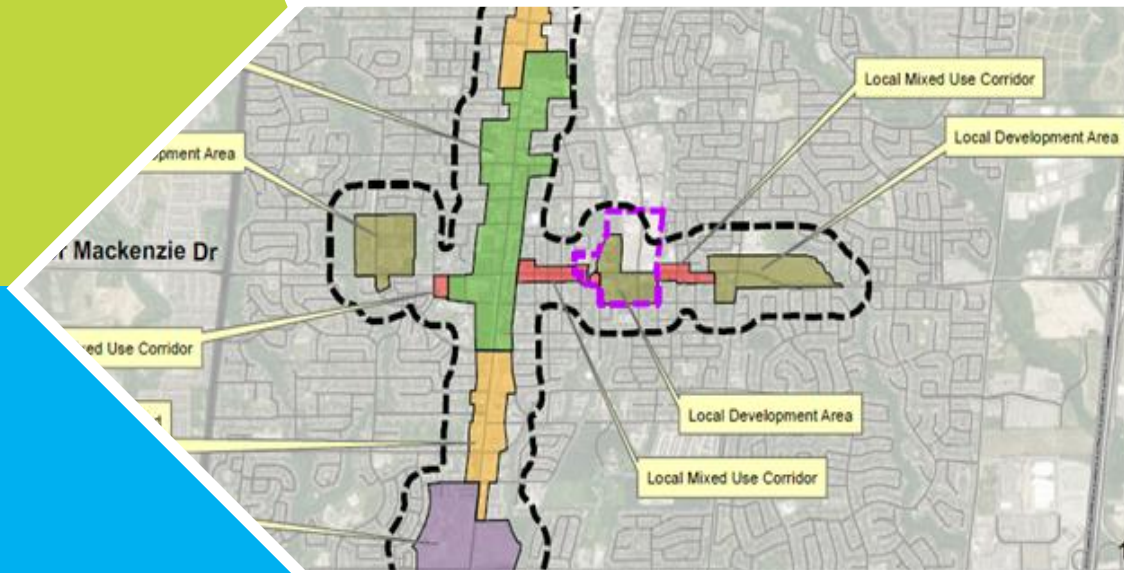


**Prepared For:
The City of Richmond Hill**

**Urban Master Environmental
Servicing Plan Update Study
Water and Wastewater Systems**



October 31, 2023

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EXECUTIVE SUMMARY

The City of Richmond Hill has completed updates to the water and wastewater components of the 2014 Urban Environmental Master Servicing Plan. These updates identify the infrastructure works needed to support the City structure framework defined by the City of Richmond Hill Official Plan. This UMESP, has followed Approach #2 under the Municipal Class Environmental Assessment process (MCEA, October 2000). This Study documents the completion of Phases 1 and 2 of the EA process. The UMESP recommends a series of projects intended to address the following Problem Statement:

The Update to the Urban Environmental Servicing Plan is essential to align with the Official Plan. This UMESP will provide strategic guidance for essential upgrades in municipal water and wastewater infrastructure for the 2041, 2051 and Ultimate Build-Out growth scenarios, all of which are necessary to facilitate and sustain urban growth and intensification as outlined in the City's Official Plan.

The study focuses on the Richmond Hill Centre, Key Development Areas, Regional Corridors, Local Centres, Local Development Areas, and Local Corridors, as defined by the Official Plan and roughly extending the length of the Yonge Street corridor through the City.

The City currently owns and operates approximately 9,292 sanitary sewers and 8,928 sanitary maintenance holes. The sewer diameters range from 125 mm to 2,700 mm. The City's wastewater collection system discharge to four Regional trunk sewers: the North Don Collector, the Central Collector, the Richmond Hill Collector (also referred to as the Yonge Street Collector) and the 19th Avenue Collector (which flows east to the YDSS).

Performance assessment of the existing wastewater collection system was completed as part of this study. To assess the system performance, a wastewater collection system hydraulic model was updated and utilized. The model was updated using the most recently available data and calibrated using flow and rainfall data collected at twenty-one (21) strategic flow monitoring sites and three (3) rain gauges. Collected data was reviewed and utilized to calibrate and validate the model in accordance with industry best practices. Collected data was also used to assess the extent of inflow and infiltration (I/I) into the wastewater collection system.

The existing wastewater system performance was assessed for dry weather flow conditions and wet weather flow conditions (25-year design storm event). For the 25-year design storm event, a significant number of local sewers were identified to surcharge due to bottlenecks and backwater, caused by bottlenecks. A level of surcharge (water level above the obvert of the pipe) greater than 0.3 m can be seen along Naughton Drive, East of Yonge Street. Also, at multiple locations between Elgin Mills Road and Major Mackenzie Drive; and along Harding Boulevard, just south of Major Mackenzie Drive. In consultation with the City, if surcharge is present during existing conditions, and is not caused by future growth within the Study Area, then remedial measures will not be evaluated as part of this study. As expected, the level of surcharge gradually increases for each future scenario (2041, 2051 and Ultimate Build-out growth scenarios).

The evaluation of alternatives concluded that enhancement and expansion of the existing collection system be undertaken. The analysis identified twenty (20) improvement projects to accommodate Ultimate Build-Out growth, with the majority of these being required by the 2051 growth projections within the study area. The total cost of the recommended wastewater system improvements was estimated to be \$41.1M.

The City's water distribution system is a part of the York Region's system. The City provides water to its customers through four (4) Pressure Districts (PD6, PD7, PD8, and PD9, respectively). The existing conditions model reflects the system's condition to 2016. There were some changes in the system since 2016 that were only applied in the future conditions model. For example, the Flow Modulated Area (FMA) was removed in 2017. The valve setting in FMA was changed in 2017 which applied only to the future condition model.

The water model was run under 2016 Maximum Daily Demand and Peak Hour Demand. The modelling results are generally within 100 kPa of the hydrant test results. However, there are some areas with modeled pressures more than 100 KPa difference with the hydrant test data. Results outside of 100 KPa range tend to be clustered, which could indicate operational issues that aren't modelled correctly (partially-closed valves or possibly nearby watermains temporarily closed for operational/maintenance purposes), or hydrant test data that was not undertaken during the daily peak demand hour.

A total of fifteen (15) improvement projects were identified for the City's water distribution system to accommodate the projected growth through 2041, 2051, and Ultimate Build-Out. All projects are triggered by fire flow insufficiencies based on the recommended fire flow demands and designated land use and built form of the future developments in the intensification areas. Main improvement areas include new watermains on Yonge Street corridor and watermain replacements on side streets west of Yonge Street to service future developments, as well as 1,410 m of watermain replacements on Church Street in the Downtown Local Centre. Other areas include watermain replacements in Local Development Areas south of Major Mackenzie Drive East as well as in Richmond Hill Centre. Thirteen (13) of the fifteen (15) projects are required to be implemented before 2041 either fully or partially with only two (2) projects required after 2041. The total cost of the proposed projects is \$ 21,668,980 of which \$ 16,030,537 required before 2041.

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Appendices

Appendix A - Technical Memorandum: Wastewater Model Development and Results

Appendix B – Technical Memorandum: Water Model Development and Results

Appendix C – Public Consultation Materials

1.0 Introduction

1.1 Background Review

In 2010, Richmond Hill Council adopted a new Official Plan (OP) that sets out the long-term vision for growth and development in Richmond Hill. Given that Richmond Hill's settlement area is nearly built out, most of the future growth and development in the City will occur through intensification. Directing new growth to the centres and corridors as set out in the Plan represents a city-building approach to developing a complete community. The hierarchy of centres and corridors is intended to achieve the City's intensification target and accommodate growth based on a comprehensive land use planning framework to direct and manage population and employment growth to 2051.

In 2014, the City completed an Urban Master Environmental Servicing Plan (UMESP). The UMESP was undertaken to identify the infrastructure work needed to support the planned growth and intensification within centers and corridors identified in the City's Official Plan.

1.2 Objectives

In accordance with the Terms of Reference, this Urban Master Environmental Servicing Plan (UMESP) Update Study has been undertaken to update the water and wastewater components of the 2014 UMESP based on 2041, 2051 and Ultimate Build-Out growth forecasts based on the new Official Plan. This Study provide guidance to new Secondary Plans within the City's intensification centers and corridors with emphasis on the following areas of the City structure: Richmond Hill Centre, Key Development Areas, Regional Corridors, Local Centres, Local Development Areas, and Local Corridors.

This UMESP sets out environmental servicing requirements for future development applications. In addition, the UMESP is intended to identify necessary upgrades to the municipal water and wastewater infrastructure within the study area to accommodate growth, including costing to support development charge bylaws.

1.3 Class Environmental Assessment Process

As required under the Ontario Environmental Assessment Act (R.S.O. 1990), this study followed the MCEA planning process. The Act is a provincially legislated document that governs all public undertakings that have the potential to affect the environment. In this sense, the term "environment" is broadly applied to include the natural, social, cultural, constructed and economic environments. The Environmental Assessment Act defines a planning and design process that proponents of projects must follow to ensure that all environmental impacts are considered, and any effects appropriately mitigated, before their project is implemented.

The Municipal Class Environmental Assessment (Class EA) process was developed by the Municipal Engineers Association as a means to categorize proposed typical municipal projects according to their anticipated environmental impact. The Ministry of the Environment has formally recognized the Municipal Class EA process as meeting the requirements of the Environmental Assessment Act, and the MEA has issued a comprehensive document to detail the process. Key principles of successful environmental assessment planning include:

- Consultation with stakeholders and affected parties upon study commencement, and throughout the process of the project.

- Consideration of all reasonable alternatives, including “alternatives to” and “alternative methods” of implementing a preferred solution.
- Identification and consideration of broad environmental effects, as identified previously, for each alternative under evaluation.
- The systematic evaluation of all alternative solutions and/or methods to determine the net environmental effects, based on available information.
- The provision of clear and comprehensive documentation that demonstrates how the MCEA planning process was followed, and to ensure transparency and traceability of the decision-making process for the project.

1.4 Class EA Master Plans

Master Plans typically differ from project-specific studies in several key respects. Long range infrastructure planning enables the proponent to comprehensively identify needs and establish broader infrastructure options. The combined impact of alternatives is also better understood which may lead to other and better solutions. In addition, the opportunity to integrate with land use planning enables the proponent to look at the full impact of decisions from a variety of perspectives. The following are distinguishing features of Master Plans:

- a. The scope of Master Plans is broad and usually includes an analysis of the system in order to outline a framework for future works and developments. Master Plans are not typically undertaken to address a site-specific problem.
- b. Master Plans typically recommend a set of works which are distributed geographically throughout the study area and which are to be implemented over an extended period of time. Master Plans provide the context for the implementation of the specific projects which make up the plan and satisfy, as a minimum, Phases 1 and 2 of the Class EA process. Notwithstanding that these works may be implemented as separate projects, collectively these works are part of a larger management system. Master Plan studies in essence conclude with a set of preferred alternatives and, therefore, by their nature, Master Plans will limit the scope of alternatives which can be considered at the implementation stage.

This Study is being undertaken in accordance with Approach #2, as described in Appendix 4 of the MCEA document. An overview of the Municipal Class Environmental Assessment process is provided in **Figure 1-1**. This approach involves the preparation of a Master Plan document upon the completion of Phase 1 and high-level analysis and public consultation completed as part of Phase 2 of the process. The Master Plan document is then made available for public comment prior to being approved by the municipality. Under Approach #2, the Master Plan is done at a high level of assessment.

1.5 Problem and Opportunity Statement

The Problem Statement for this Class EA Master Plan has been articulated as follows:

The Update to the Urban Environmental Servicing Plan is essential to align with the Official Plan. This UMESP will provide strategic guidance for essential upgrades in municipal water and wastewater infrastructure for the 2041, 2051 and Ultimate Build-Out growth scenarios, all of which are necessary to facilitate and sustain urban growth and intensification as outlined in the City's Official Plan.

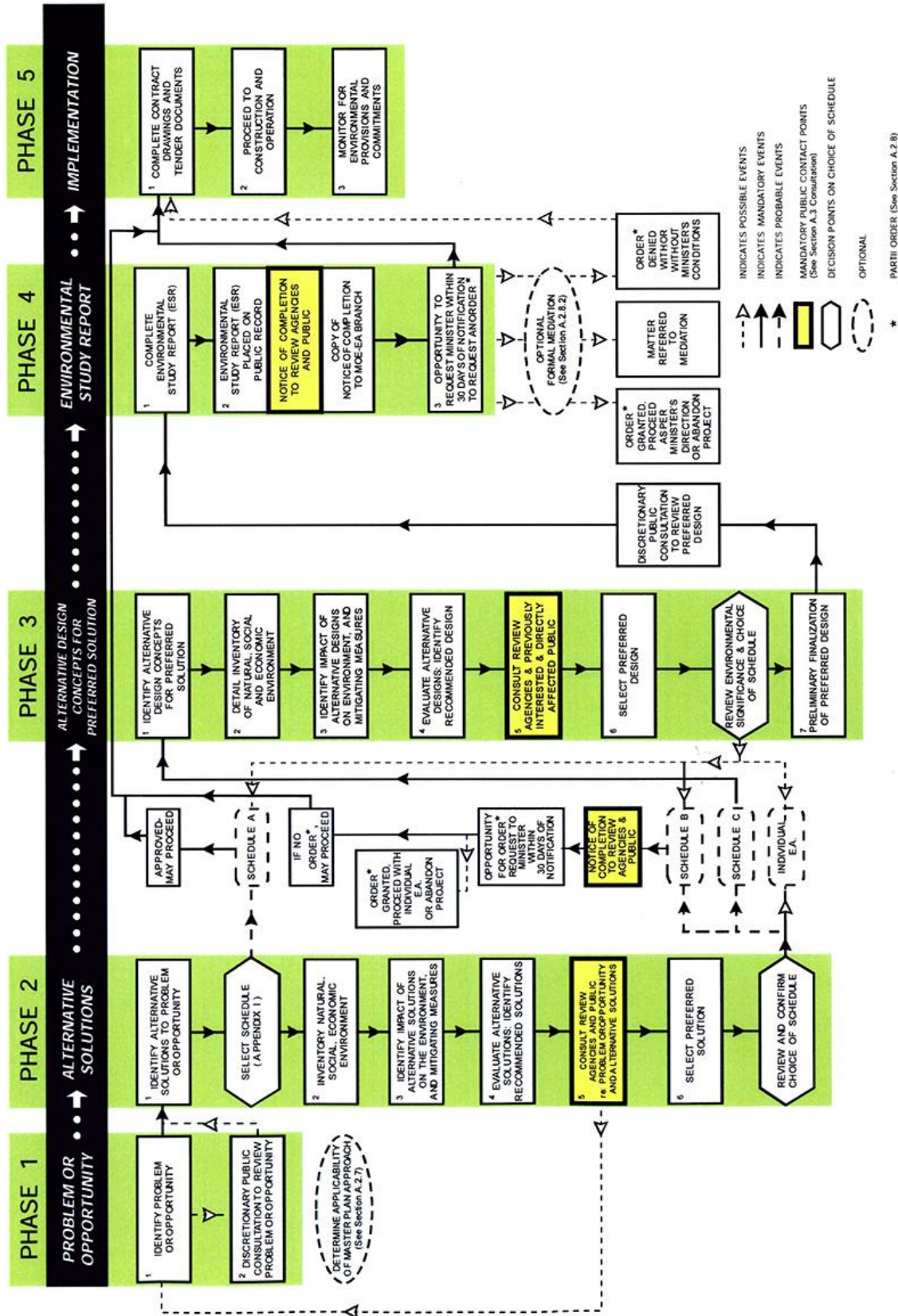


Figure 1-1: Municipal Environmental Assessment Process

1.6 Study Area

The study area comprises the network of centres and corridors which include the secondary plan areas for local centres, key development areas, and the Richmond Hill Centre urban growth centre, as well as the other remaining intensification areas identified within the Richmond Hill Official Plan. This UMESP update study area extends beyond these City structure elements, where necessary, to ensure that the analyses and infrastructure recommendations account for any interdependencies with adjacent areas. **Figure 1-2** shows the study area of this project which consists of seven Official Plan (OP) land use intensification Areas in addition to three Emerging Growth Centres (EGC):

OP Intensification Areas:

- 1) Village Local Centre
- 2) Key Development Areas
- 3) Local Development Areas
- 4) Local Mixed Use Corridor
- 5) Oak Ridges Local Centre
- 6) Regional Mixed Use Corridors
- 7) Richmond Hill Centre

Emerging Growth Centres:

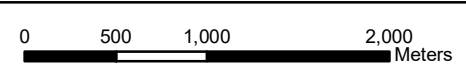
- 1) Newkirk GO Local Centre
- 2) Bathurst and Highway 7 Centre
- 3) East Beaver Creek and Highway 7 Centre.

The existing land use within the study area is comprised of a broad mix of commercial, institutional, and residential uses, interspersed by the City's Greenway system, other natural features, highways, and rail corridors. The study area primarily centres on the Yonge Street Regional corridor, which is a spine for development both within and beyond the City's boundaries.



Figure 1-1: Study Areas

Drawn By: J.H. Date: Oct 28, 2023



2.0 Background Information

2.1 2014 Urban Master Environmental Servicing Plan

In 2014, the City completed its Urban Master Environmental Servicing Plan (UMESP). The UMESP was undertaken to identify the infrastructure work needed to support the planned growth and intensification within centers and corridors identified in the City's Official Plan.

For water and wastewater services, the population and employment increases forecasted in the Official Plan will yield greater water demands and increased wastewater generation rates. Scenarios included the 2031 Official Plan planning horizon, along with 2051 and Ultimate scenarios. The Ultimate scenario represented full buildout of all parcels within the study area to the approved densities provided in the Official Plan.

Monitoring of the wastewater collection system was undertaken to compare observed flows in the sanitary sewer system to current design criteria at the time of the study commenced. The monitoring program demonstrated that observed flows are generally less than expected flows. This information did not impact the recommended improvements and may be used by the City to refine the timing of recommended improvements.

The modelling of the wastewater collection system revealed a number of deficiencies under future conditions. The evaluation of alternatives concluded that enhancement and expansion of the existing collection system be undertaken, along with consideration for long term water conservation strategies.

The analysis identified 18 improvement projects, with the majority of these being required beyond the 2031 growth projections within the study area. The total cost of the recommended wastewater system improvements was estimated to be \$24,700,000.

The modelling of the water distribution network revealed sections that do not have sufficient capacity for the future conditions. The evaluation of alternatives concluded that a single improvement project should be undertaken. The recommended improvement involves a pressure zone conversion near Yonge Street and Major Mackenzie Drive. This improvement is partially located within the Downtown Local Centre Secondary Plan Area. Street network refinements within the Secondary Plan areas may trigger the need for service improvements. The total cost of the recommended water system improvement was estimated to be \$600,000.

For this study, the 2014 UMESP was reviewed to provide strategic guidance for essential upgrades in municipal water and wastewater infrastructure for the 2041, 2051 and Ultimate Build-Out growth scenarios, all of which are necessary to facilitate and sustain urban growth and intensification as outlined in the City's Official Plan.

2.2 Richmond Hill Official Plan

The Official Plan describes a City structure framework to direct growth to built-up urban areas with existing infrastructure and services in a network of centres and corridors and determine land use spatially throughout the City.

Given that Richmond Hill's settlement area is nearly built out, most development in the City will occur through *intensification*. Intensification is the development of a property, site or area at a higher density

than currently exists. The majority of intensification in the City shall occur in the *centres and corridors* shown on Schedule A1 (City Structure) and defined by the land use designations shown on Schedule A2 (Land Use) and the policies of the Plan. Intensification outside of the centres and corridors shall be limited in accordance with the policies of the Plan.

In the City's intensification hierarchy, Centres are recognized as focal points and destination areas where a mix of uses shall be required, whereas Corridors play a supporting role to these Centres. Corridors provide connections, continuity, and transition between Centres, forming a cohesive network of mixed-use lands that serve the broader community and Region as a whole. **Figure 2-1** shows the City structure elements defined in the City's OP. The City centres and corridors identified in the City's structure are:

City Centres:

- Richmond Hill Centre
- Yonge Street and 16th Avenue/Carrville Road Key Development Area
- Yonge Street and Bernard Avenue Key Development Area
- Village Local Centre
- Oak Ridges Local Centre
- Newkirk Local Centre
- Bathurst and Highway 7 Local Centre
- East Beaver Creek and Highway 7 Local Centre
- Trench Street Local Development Area
- Bayview Local Development Area

City Corridors:

- Yonge Street Regional Corridor
- Highway 7 Regional Corridor
- Major Mackenzie Local Corridor

Richmond Hill Centre: Richmond Hill Centre at Yonge Street and Highway 7 is identified as an urban growth centre in the Growth Plan for the Greater Golden Horseshoe and as a Regional Centre in the York Region Official Plan. The Richmond Hill Centre is part of the Richmond Hill/Langstaff Gateway Urban Growth Centre (UGC) shared with the City of Markham. It will become a vibrant, urban mixed-use centre that is transit-oriented and supports pedestrian and other forms of active transportation. This centre will contain the greatest height and densities in the City, focused around a major inter-modal Regional transit hub.

Key Development Areas (KDAs): KDAs are intensification areas located along the Regional Corridors where public rapid transit services intersect with major nodes of retail and commercial development, and where opportunities exist for redevelopment of large land parcels that can support new public streets. The Yonge and 16th Avenue KDA and the Yonge and Bernard Avenue KDA are envisioned as sub-centres or inter-modal nodes between the Local and Regional Centres.

Regional Corridors: Both Yonge Street and Highway 7 are Regional Corridors. Regional Corridors function as key connections between centres in York Region and centres across the Greater Toronto Area (GTA), accommodating rapid transit. While the Regional Corridors are anticipated to accommodate intensification, this Plan recognizes that the character of the areas along the corridors vary (i.e., historical

character, natural heritage, employment areas). Certain portions of the Regional Corridors will not develop or intensify in order to protect the Greenway System, to be maintained for employment uses only, and/or to maintain its predominant neighbourhood function.

Local Centres: Local Centres are intended to function as mixed use centres to serve adjacent neighbourhoods. They will be transit supportive, complete community areas, each with a unique character due to the existing built context, cultural heritage and/or natural heritage that exists within them. Five Local Centres (Village, Oak Ridges, Newkirk, Bathurst/Highway 7, and East Beaver Creek/Highway 7) are identified in the City's structure.

Local Development Areas (LDAs): LDAs are areas where a cluster of land uses and/or public transit services intersect with a Local Corridor providing a local destination point and opportunities for redevelopment. The Trench Street LDA and the Bayview LDA are envisioned as small-scale, sub-centres along the Local Corridor to serve adjacent neighbourhood and the community.

Local Corridors: Major Mackenzie Drive is identified as a Local Corridor. The Local Corridor functions as a key east-west connection between municipalities in southern York Region and is planned to accommodate a public rapid transit system. While the Local Corridor is anticipated to accommodate intensification, this Plan recognizes that the character of the corridor varies along its length in terms of building form, land use, and intensification of land uses.

Remaining areas, including Employment Lands, are not projected to undergo the level of intensification associated with the above noted elements of the City structure, and fall outside the study area scope of this MESP.

On the contrary, the Greenway System, which is defined as a key element of the City's City structure, is relevant to this MESP as development and associated infrastructure works must protect and enhance the Greenway System.

The Official Plan also sets out the densities associated with the various land use types. These densities were used to project beyond the 2041 forecast to identify whether additional municipal service constraints would ensue for the ultimate buildout, beyond 2051. These scenarios have been evaluated to provide the City with the ability to adapt to the potential variability of redevelopment patterns and timing.

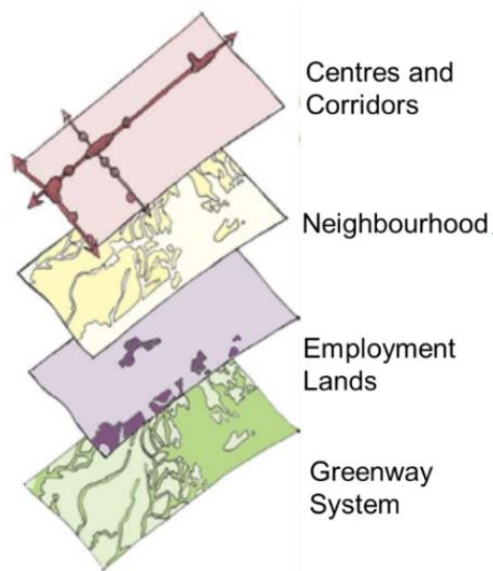


Figure 2-1: City structure Elements. Source: Key Directions Report

2.3 York Region Official Plan

The York Region Official Plan was approved by the Province of Ontario with modifications in November 2022, and provides the basis for the delivery of infrastructure and facilities needed to support the established growth projections, which are consistent with those defined by Richmond Hill's Official Plan.

This UMESP addresses the City infrastructure and facilities needed to support projected growth, but coordination with the Region was undertaken to ensure that interdependencies were thoroughly considered in the identification of proposed works and improvements.

2.4 York Region Water and Wastewater Master Plan

York Region's water and wastewater infrastructure systems support and supply the local City of Richmond Hill water and wastewater systems. The Region's Master Plan Update was endorsed by Regional Council in May 2022, and considered the improvements needed to service the growth projections outlined in both the City's and Region's Official Plans to the year 2051 and beyond.

The Region's Master Plan and corresponding infrastructure data were reviewed in relation to this UMESP to coordinate with the evaluation of the City's water and wastewater networks.

2.5 Study Components and Requirements

In terms of support for planning processes, each component of this UMESP sets out to address the infrastructure improvements, requirements, and recommendations necessary at three different scales:

- At the **study area wide scale**, the recommendations are associated with the overall study area;
- At the **Secondary Plan scale**, the recommendations provide input and guidance with respect to the infrastructure needs for those areas within the UMESP study area requiring Secondary

- Plans. As required in the Official Plan, Secondary Plans will be completed for the Richmond Hill Centre, the Downtown Local Centre, the Oak Ridges Local Centre, the Yonge Street and 16th Avenue KDA, and the Yonge Street and Bernard Avenue KDA.
- At the **Site Plan scale**, the recommendations provide requirements and guidelines for infrastructure works, criteria, and analyses that must accompany individual site-specific development applications.

A steering committee for this UMESP was established at the outset and included representatives of a number of City departments, York Region, and members of the study team.

3.0 Existing Environment

3.1 *Natural Environment and the Greenway System*

As established by the Environmental Policy Review, the Greenway System is the overarching framework for the City's natural heritage policies and will not accommodate intensification and only those uses outlined under the Greenway System land use designations will be permitted. **Figure 3-1** illustrates the City of Richmond Hill Greenway System in the vicinity of the study areas as depicted on Schedule A1 from the Official Plan.

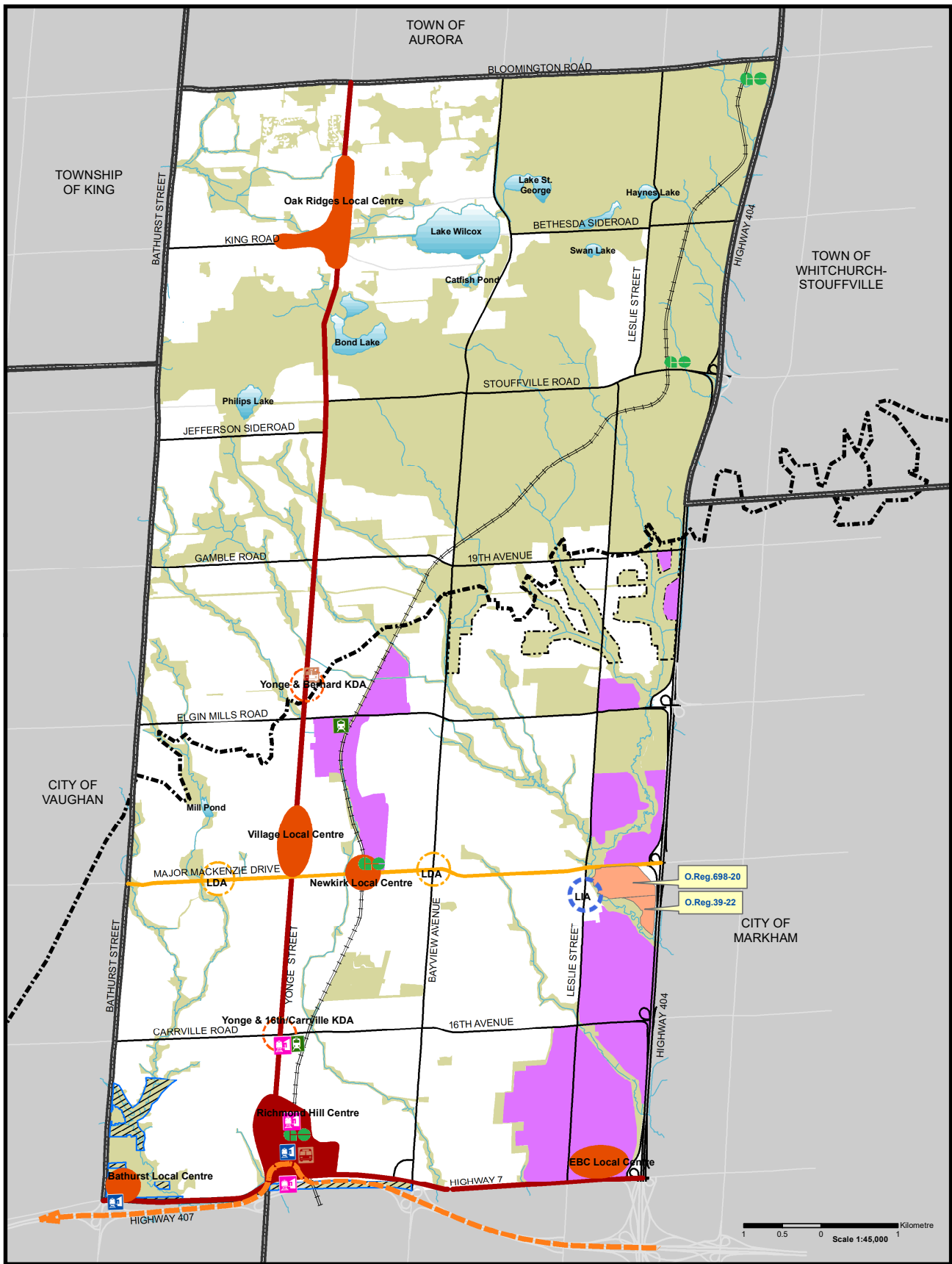


FIGURE 3-1:
RICHMOND HILL
OFFICIAL PLAN
City Structure
SCHEDULE A1

Legend

- Centres & Corridors**
- Richmond Hill Centre
 - Key Development Areas
 - Local Centres
 - Local Development Areas
 - Leslie Street Institutional Area
 - Regional Corridor
 - Local Corridor

- Neighbourhood Areas
 - Employment Lands
 - Greenway Systems
- Transit Infrastructure**
- Existing GO Station
 - Proposed GO Station
 - Terminal
 - Proposed TTC Subway

- 407 Transitway
- Proposed 407 Transitway Stations
- Oak Ridges Moraine Conservation Plan Area
- Greenbelt Plan Area
- Parkway Belt West Plan
- Areas Subject to Minister's Zoning Order
- Waterbodies
- Watercourses
- C.N.R.

NOTE: The information provided in this Schedule constitutes an operative part of the Richmond Hill Official Plan. While every effort is made to ensure accuracy, currency and completeness, it is not a plan of survey. Due to size constraints and changes that occur over time, the City cannot warrant its accuracy, currency and completeness. Interested parties are therefore urged to make enquiries with the City of Richmond Hill Planning and Infrastructure Department to ensure that the information depicted in this Schedule is accurate, current and complete in all respects.



The Greenway System occurs in both the urban and rural areas of the City. It is generally comprised of rural, agricultural and environmental lands. Urban open spaces also form part of the interconnected Greenway System. The Greenway System provides tourism and economic opportunities as well as social/recreational and environmental opportunities. The natural areas of the Greenway System include features and functions of the natural environment including a portion of the Oak Ridges Moraine, the Greenbelt urban river valleys, the Don River, Rouge River, Humber River, and East Holland River system, and numerous wetlands, kettle lakes, forests, and woodlots in the City. This Official Plan protects and enhances the Greenway System over the long-term.

3.2 Socio-Economic Environment

The City serves as both a destination and a major transit hub for commuters in the GTA. Socio-economic, natural and cultural landmarks such as the historic village core in the Downtown, the identifiable Beaver Creek Business Park, neighborhoods such as Mill Pond and Lake Wilcox, and the David Dunlap Observatory continue to provide unique and interesting places. Foremost, the City's central location in the middle of a thriving metropolitan region places Richmond Hill front and centre in the GTA. This location is partly what led to the City's past growth and development and will continue to influence it in the future.

In recognition of the City's urbanization and limited supply of employment lands, a comprehensive approach is set out to provide for an appropriate mix of employment uses within the City including industrial, commercial, retail and institutional uses to meet long term needs. The future economic vision for the City is for continued strong economic performance on its employment lands with an increased emphasis on developing its centres and corridors as the focal points of economic activity for office and population-related employment. To maintain and promote Richmond Hill's economic vitality, this Official Plan protects and seeks to maximize the appropriate use for the City's employment lands while diversifying Richmond Hill's economic base, where appropriate, throughout the City.

The City's local economy is well-served by its business parks along the Highway 404 corridor and within the Newkirk Business Park. The City has experienced pressure to convert its employment lands to retail, commercial, residential and other non-employment uses. Recognizing the City's limited employment land supply, this Plan maintains and protects the City's existing employment lands for employment uses as permitted by the policies of this Official Plan. The intent of these policies is to ensure that the long-term supply of employment lands is maintained to meet the future needs of the City and to ensure an appropriate balance of land uses across the City.

4.0 Water and Wastewater Services

4.1 General

This portion of the UMESP document outlines the current status of the wastewater collection and water distribution systems in the UMESP study region. By constructing models for these systems using the InfoWorks ICM and InfoWater platforms, this assessment forms the foundation for identifying any limitations in servicing that might impact future development in the study area, and for choosing the suitable strategies to tackle these limitations.

The future condition projections applied to the study area are based on the values and principles described in **Section 5.0**.

At the onset of assignment, there was an underlying assumption that future growth would need improvements to the water and wastewater infrastructure. Thus, it becomes crucial to promptly recognize the opportunities and limitations that might influence the assessment of alternative solutions, ensuring the reliability of the resulting recommendations.

4.2 Wastewater

The City of Richmond Hill currently owns and operates approximately 9,292 sanitary sewers and 8,928 sanitary maintenance holes. The sewer diameters range from 125 mm to 2,700 mm. The City's wastewater collection system discharge to four Regional trunk sewers which flow in a southerly direction, including:

- The North Don Collector;
- The Central Collector;
- The Richmond Hill Collector (also referred to as the Yonge Street Collector); and,
- The 19th Avenue Collector (which flows east to the YDSS).

For the purpose of this study, the study area was limited to the local sanitary sewers. **Figure 4-1** illustrates the local and the Regional trunk systems.

4.2.1 Wastewater Model Development

The sewer network information was initially assembled using both the sewer network database provided by the City and the previous InfoSewer model; however, some assets had one or multiple gaps or issues. Therefore, appropriate assumptions were made to rectify/infill the data gaps. See **Appendix A** for details on the data gap rectification.

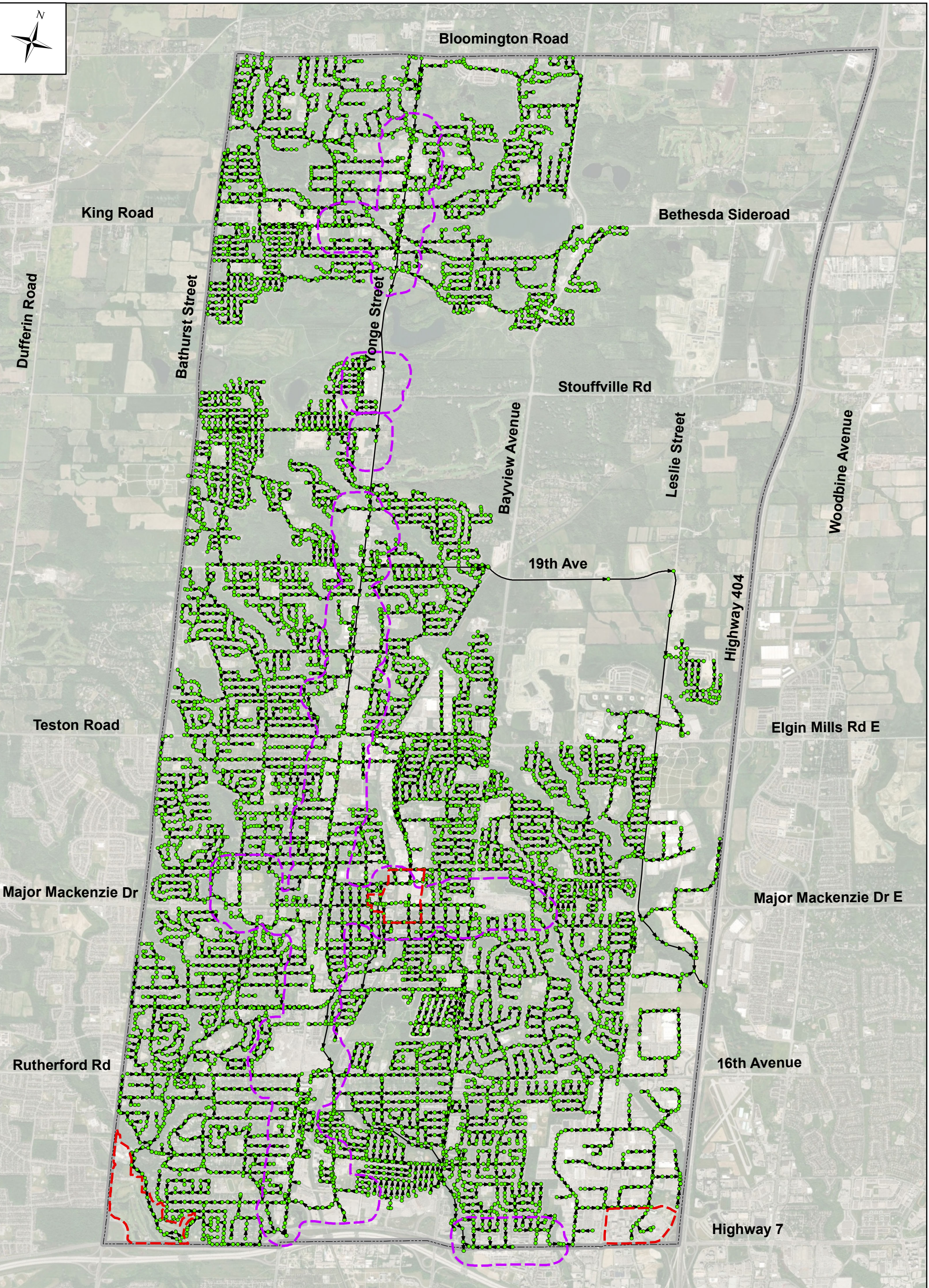
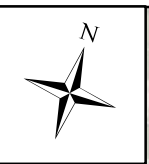
New subcatchments were discretized based on the lot fabric shapefile information, and population was assigned using the 2016 census data per Traffic Zone (TZ) and Water Consumption (WC) data. A total of 7,536 subcatchments were delineated. The 2016 annual water consumption data per individual address was used to distribute the population within each traffic zone.

For boundary conditions, the Regional trunk sewers were evaluated under the 25-year design storm (using the IDF curve pre-2021) and confirmed no surcharge conditions. Therefore, it can be concluded that there is no backflow affecting local sewers, and the downstream system can be modelled as free flow outfalls.

4.2.2 Sanitary Flow Monitoring

Available historical rainfall and flow monitoring data was reviewed and analyzed to characterize dry weather flow (DWF) and wet weather flow (WWF) for use in the hydrodynamic model calibration. The flow monitoring data provides an indication of the performance of the wastewater network under various rain events.

Flow data from twenty-one (21) sanitary flow monitoring stations and three (3) rain gauges were selected for this study. **Figure 4-2** shows the location of these flow meter stations and rain gauges.



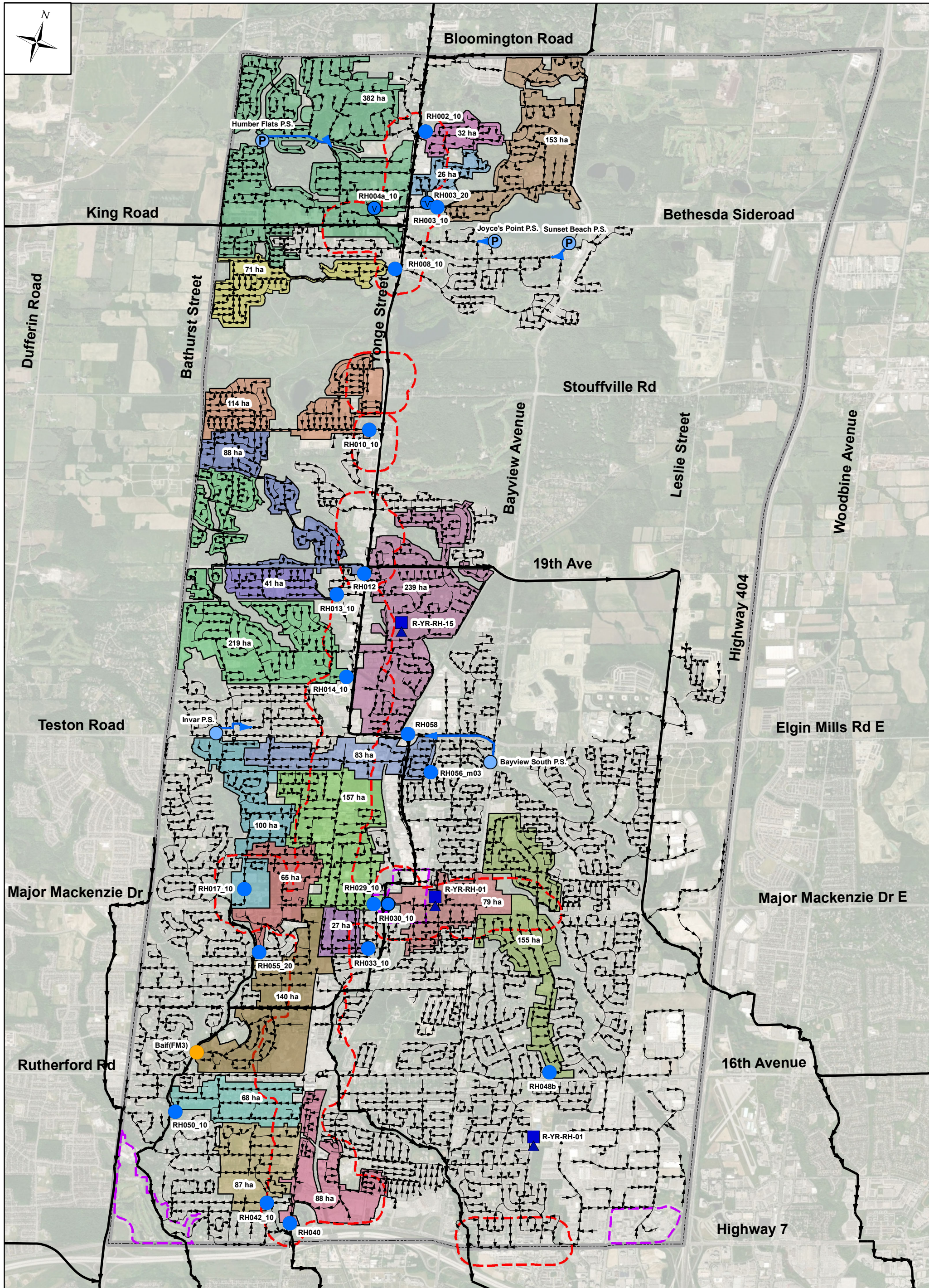
Legend

- Existing Sanitary Manholes
- Existing Sanitary Conduits
- Study Boundary
- Emerging Growth Centres
- Municipal Boundary

**Figure 4-1:
Wastewater Collection System**

Drawn By: J.H. Date: Oct 28, 2023





Dufferin Road

King Road

Bathurst Street

Yonge Street

Bloomington Road

Bethesda Sideroad

Stouffville Rd

Bayview Avenue

Leslie Street

Woodbine Avenue

19th Ave

Highway 404

Teston Road

Elgin Mills Rd E

Major Mackenzie Dr

Major Mackenzie Dr E

Rutherford Rd

16th Avenue

Highway 7



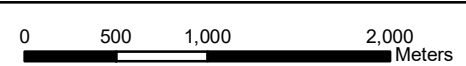
RIC18-0004 -
Richmond Hill UMESP Update

Legend

- 2016 Flow Monitoring Locations (20)
- 2017 Flow Monitoring Location (1)
- ▲ 2016 Rain Gauges (3)
- P Existing Pumping Stations (5)
- Forcemains
- Sanitary Sewers
- York Region Sanitary Trunk Sewers
- Drainage Area
- Study Areas
- Emerging Growth Centres
- Municipal Boundary

**Figure 4-2:
Drainage Areas and Flow
Monitoring Locations
(2016 & 2017)**

Drawn By: J.H. Date: Oct 28, 2023



4.2.2.1 Dry Weather Flow

The measured flow parameters for each subcatchment are summarized in **Table 4-1**. These values were calculated using data from the 2016 flow monitoring period. The population is estimated based on the 2016 Census and Traffic Zone given by the City.

Table 4-1: DWF Parameters used in the model

Flow Monitoring Station	Area (ha)	Land Use ⁽¹⁾	Residential Population	ICI Population	Total Population	Daily DWF Volume (L)	DWF Rate (L/c/d)	Measured Peaking Factor
RH002_10	31.6	Res	954	38	992	320,475	323	1.64
RH003_10	134.3	Res	4,957	206	5,163	1,403,637	272	1.34
RH003_20	26.2	Res	933	232	1,165	327,971	282	1.33
RH004a_10	366.9	Res	11,920	1,620	13,540	3,737,550	276	1.31
RH008_10	69.2	Res	3,979	-	3,979	990,285	249	1.49
RH010_10	111.7	Res	6,148	391	6,539	1,025,462	157	1.83
RH012	83.5	Res	3,921	427	4,348	996,909	229	1.59
RH013_10	38.8	Res	2,198	302	2,500	679,759	272	1.61
RH014_10	213.8	Res	11,109	519	11,628	3,106,695	267	1.7
RH058	221.3	Res	11,739	1,569	13,308	2,310,197	174	1.42
RH056_m03	82.2	Res + ICI	1,922	1,684	3,606	298,930	83	1.51
RH017_10	99.7	Res + ICI	2,275	3,124	5,399	1,059,969	196	1.39
RH055_20	65.0	Res	1,922	116	2,038	814,968	400	1.21
RH029_10	157.4	Res + ICI	5,667	3,141	8,808	2,048,231	233	1.25
RH030_10	78.7	Res	3,328	777	4,105	1,284,364	313	1.34
RH033_10	27.1	Res	1,238	246	1,484	341,250	230	1.31
RH050_10	67.8	Res	1,499	359	1,858	731,657	394	1.17
RH040	88.3	Res + ICI	6,889	2,533	9,422	1,988,368	211	1.48
RH042_10	70.5	Res + ICI	1,594	460	2,054	646,910	315	1.26
RH048b	151.5	Res	5,993	1,306	7,299	1,671,329	229	1.55
Civica FM3	139.8	Res + ICI	5933	3744	9677	2,212,638	223	1.23

⁽¹⁾ Areas with 80% or more residential land use were assumed to be Residential (RES), areas with 80% or more ICI land use were assumed to be ICI, the rest were assumed to be mixed-used (Res + ICI)

4.2.2.2 Wet Weather Flow

The number and magnitude of significant storms is important for assessing the suitability of the data for model calibration. The greater the number and larger the magnitude of storms, the more reliable and accurate the model should be when used to predict system response during critical (design) storms. **Table 4-2** summarizes the most intense storms captured during the 4-months period, from May 1, 2016 to August 31, 2016, selected to calibrate the model.

Table 4-2: The 5 most intense storms captured during the 4-months monitoring period

Event	R YR RH 01 Peak Intensity 5 min (mm/hr)	R YR RH 10 Peak Intensity 5 min (mm/hr)	R YR RH 15 Peak Intensity 5 min (mm/hr)	Return Period
August 13, 2016	74.4	55.2	84.0	< 2-year
July 25, 2016	64.8	62.4	62.4	< 2-year
June 11, 2016	55.2	62.4	52.8	< 2-year
July 27, 2016	50.4	0	43.2	< 2-year
June 5, 2016	38.4	19.2	26.4	< 2-year

4.2.3 Wastewater Model Calibration

The Groundwater Infiltration Module (GIM) method was used to calibrate the model for wet weather responses. The advantage of the InfoWorks GIM is to provide a large, highly attenuated response to rainfall which typically originates from groundwater infiltration, or large pervious areas of the catchment, as well as to allow continuous simulation.

There is a total of ten (10) parameters used in GIM. Of these, the Porosity of Soil, Porosity of Ground and the Soil Depth should be fixed as they have a physical basis. The remaining seven (7) parameters are calibration coefficients and these should be adjusted to match the observed flows. The continuous simulation accounts for antecedent moisture conditions (AMC) which may have an impact on the I/I response.

4.2.4 Calibration Results

A 4-month period, from May 1, 2016 to August 31, 2016 was selected to calibrate the City’s sanitary sewer. The results show good agreement between the measured and modelled values for all stations, except for station RH003_20, as shown in **Table 4-3**. For station RH003_20, the predicted peak flow for the August 14-19 event is 33% higher than the observed peak flow. The results shows that all the other simulated events were within the WaPUG criteria in terms of both peak flow and total volume. This confirms that the model is able to project the actual scenario within an acceptable deviation.

Further information about the wastewater model development, calibration and results can be found in **Appendix A**.

Table 4-3: Model calibration results

Station	Event Date	Rainfall		Peak Flow (m3/s)			Volume (m3)		
		Volume	Peak Intensity (mm/hr)	Measured	Modelled	Difference (%)	Measured	Modelled	Difference (%)
RH002_10	Aug 14-19 2016	21.0	14.4	0.010	0.009	-10%	1,739	1,681	-3%
	July 24-30 2016	41.2	62.4	0.009	0.011	22%	1,962	2,098	7%
	May 10-20 2016	26.4	24.0	0.009	0.008	-11%	3,217	3,324	3%
RH003_10	Aug 14-19 2016	21.0	14.4	0.032	0.035	9%	7,742	7,329	-5%
	July 24-29 2016	30.8	62.4	0.024	0.029	21%	6,312	7,242	15%
	June 03-13 2016	30.2	52.8	0.029	0.026	-10%	15,419	14,717	-5%
RH003_20	Aug 14-19 2016	21.0	14.4	0.006	0.008	33%*	1,625	1,700	5%
	July 24-27 2016	18.8	62.4	0.005	0.006	20%	1,170	1,223	5%
	June 03-13 2016	30.2	52.8	0.007	0.006	-14%	3,518	3,418	-3%
RH004a_10	Aug 14-19 2016	21.0	14.4	0.080	0.100	25%	19,347	19,312	0%
	July 13-15 2016	14.2	21.6	0.064	0.064	0%	9,998	10,498	5%
	June 03-13 2016	30.2	52.8	0.079	0.073	-8%	41,593	38,059	-8%
RH008_10	June 03-13 2016	30.2	52.8	0.020	0.020	0%	9,838	10,375	5%
	May 12-18 2016	26.4	24.0	0.020	0.020	0%	7,023	7,227	3%
	July 01-26 2016	42.8	62.4	0.019	0.018	-5%	26,054	27,253	5%
RH010_10	Aug 15-19 2016	21.0	14.4	0.029	0.028	-3%	4,704	4,409	-6%
	June 03-13 2016	30.2	52.8	0.028	0.025	-11%	10,243	10,808	6%
	May 09-21 2016	26.4	24.0	0.027	0.026	-4%	12,664	12,639	0%
RH012	Aug 14-19 2016	21.0	14.4	0.020	0.023	15%	4,800	5,125	7%
	July 24-30 2016	41.2	62.4	0.023	0.026	13%	6,056	6,349	5%
	June 03-13 2016	30.2	52.8	0.021	0.020	-5%	10,247	10,360	1%
RH013_10	Aug 14-19 2016	21.0	14.4	0.018	0.016	-11%	3,573	3,454	-3%
	July 24-30 2016	41.2	62.4	0.018	0.021	17%	4,281	4,318	1%
	June 03-13 2016	30.2	52.8	0.014	0.014	0%	7,234	6,970	-4%
RH014_10	Aug 14-19 2016	21.0	14.4	0.073	0.069	-5%	17,209	15,999	-7%
	July 24-30 2016	41.2	62.4	0.081	0.082	1%	19,133	19,931	4%
	June 30 - July 04 2016	9.6	19.2	0.066	0.059	-11%	12,477	12,422	0%
RH058	May 11-21 2016	26.4	24.0	0.049	0.045	-8%	11,843	11,983	1%
	June 03-13 2016	30.2	52.8	0.044	0.045	2%	20,975	24,154	15%
	July 23-27 2016	31.0	62.4	0.043	0.050	16%	13,931	14,155	2%
RH056_m03	Aug 15-21 2016	15.8	14.4	0.016	0.014	-13%	2,228	2,009	-10%
	July 23-26 2016	23.0	64.8	0.016	0.015	-6%	1,424	1,446	2%
	July 27 - Aug 06 2016	19.0	50.4	0.014	0.013	-7%	3,864	3,923	2%

Station	Event Date	Rainfall		Peak Flow (m3/s)			Volume (m3)		
		Volume	Peak Intensity (mm/hr)	Measured	Modelled	Difference (%)	Measured	Modelled	Difference (%)
RH017_10	Aug 12-19 2016	53.0	74.4	0.047	0.059	26%	7,678	8,037	5%
	July 24-30 2016	37.2	64.8	0.029	0.028	-3%	7,434	6,838	-8%
	June 09-13 2016	12.2	55.2	0.020	0.019	-5%	3,608	3,723	3%
RH055_20	Aug 12-19 2016	53.0	74.4	0.039	0.046	18%	6,088	6,406	5%
	June 03-13 2016	33.4	55.2	0.016	0.016	0%	8,851	8,650	-2%
	May 16-31 2016	15.2	31.2	0.017	0.017	0%	16,343	14,960	-8%
RH029_10	Aug 12-19 2016	53.0	74.4	0.133	0.138	4%	14,726	15,855	8%
	July 24-30 2016	37.2	64.8	0.094	0.094	0%	13,134	13,481	3%
	June 03-13 2016	33.4	55.2	0.058	0.051	-12%	21,854	21,626	-1%
RH030_10	Aug 12-19 2016	53.0	74.4	0.061	0.057	-7%	9,331	9,499	2%
	July 22-31 2016	37.2	64.8	0.034	0.034	0%	11,830	11,849	0%
	June 03-13 2016	33.4	55.2	0.024	0.024	0%	13,499	13,294	-2%
RH033_10	Aug 12-19 2016	53.0	74.4	0.033	0.029	-12%	2,849	2,686	-6%
	July 24-30 2016	37.2	64.8	0.021	0.021	0%	2,088	2,271	9%
	June 03-13 2016	33.4	55.2	0.012	0.011	-8%	3,299	3,626	10%
RH050_10	Aug 12-19 2016	53.0	74.4	0.061	0.056	-8%	6,434	5,925	-8%
	July 10-20 2016	13.0	31.2	0.016	0.016	0%	7,865	7,663	-3%
	May 11-21 2016	22.8	31.2	0.016	0.018	13%	8,239	7,643	-7%
RH040	Aug 14-19 2016	15.8	16.8	0.039	0.037	-5%	10,220	10,044	-2%
	July 24-30 2016	24.8	62.4	0.040	0.035	-13%	12,317	12,403	1%
	June 03-13 2016	37.4	62.4	0.040	0.035	-13%	20,339	20,564	1%
RH042_10	Aug 12-19 2016	52.2	55.2	0.053	0.060	13%	5,107	5,057	-1%
	June 02-16 2016	37.4	62.4	0.027	0.025	-7%	9,970	9,254	-7%
	May 11-21 2016	23.0	40.8	0.017	0.016	-6%	7,953	6,336	-20%
RH048b	Aug 14-19 2016	15.8	16.8	0.037	0.036	-3%	8,526	8,717	2%
	July 24-30 2016	24.8	62.4	0.041	0.046	12%	10,458	11,037	6%
	June 03-13 2016	37.4	62.4	0.033	0.034	3%	17,141	17,842	4%
Civica FM3	July 12-17 2017	42.7	91.4	0.057	0.055	-4%	9,855	11,538	17%
	July 19-23 2017	23.9	67.1	0.054	0.067	24%	8,126	8,677	7%
	Aug 17-24 2017	38.6	103.6	0.05	0.061	22%	14,328	16,482	15%

*Predicted peak flow is not within the WaPUG criteria

4.2.5 *Model Assumptions and Limitations*

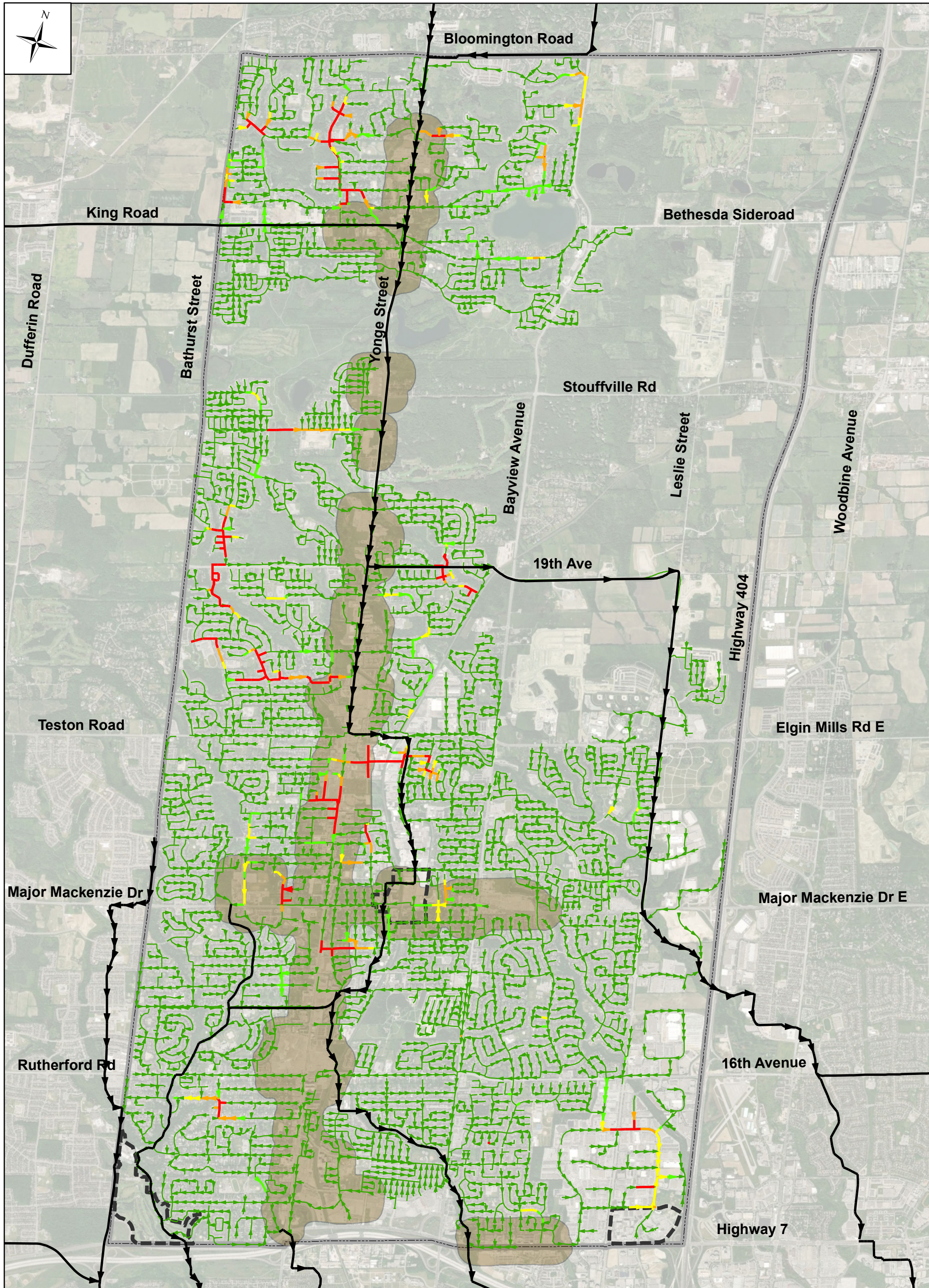
Based on the quality of the data available at the time the model was developed, and the assumptions made in consultation with the City of Richmond Hill, the wastewater hydraulic model provides the most realistic representation of the existing sewer network performance conditions. Nevertheless, it should be noted that there are some limitations with the use and application of the calibrated model for the study area. The model limitations include:

- The best possible information available at the time was used to create, calibrate and validate the model; however, assumptions had to be made to fill the data gaps;
- Flow monitoring was used to calibrate the model. For non-monitoring areas, the City's design criteria was used DWF and I/I rates;
- The model was calibrated using rainfall events with a return period less than 2-year;
- Thiessen polygons were assumed for rainfall distribution; and,
- The model is designed to predict municipal sewer surcharge, and not to detect temporary or permanent potential operational failures such as sewer blockages.

4.2.6 *Assessment of Existing Conditions*

Under existing conditions, the existing sanitary sewer performance under the City's 25-year design storm is depicted in **Figure 4-3**. As shown, the sanitary sewer system is predicted to surcharge at multiple locations due to bottlenecks and backwater, caused by bottlenecks. A level of surcharge (water level above the obvert of the pipe) greater than 0.3 m can be seen along Naughton Drive, East of Yonge Street. Also, at multiple locations between Elgin Mills Road and Major Mackenzie Drive; and along Harding Boulevard, just south of Major Mackenzie Drive. All the surcharging locations can be seen in **Figure 4-3**.

In consultation with the City, if surcharge is present during Existing Conditions, and is not caused by future growth within the Study Area, then remedial measures will not be evaluated as part of this study. Further studies are needed to evaluate proposed solutions and eliminate surcharge conditions in locations outside of the study area, and surcharge caused by existing developments (2016 population).



Legend

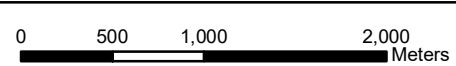
**Manhole Water Depth (m)
(Water Level to Obvert)**

- No Surcharge
- 0 - 0.15
- 0.15 - 0.3
- 0.3 - 0.6
- > 0.6

- York Region Sanitary Trunk Sewers
- Study Area Boundary
- Emerging Growth Centres
- Municipal Boundary

**Figure 4-3:
Level of Surcharge Under
Existing Conditions**

Drawn By: J.H. Date: Oct 28, 2023



4.3 Water

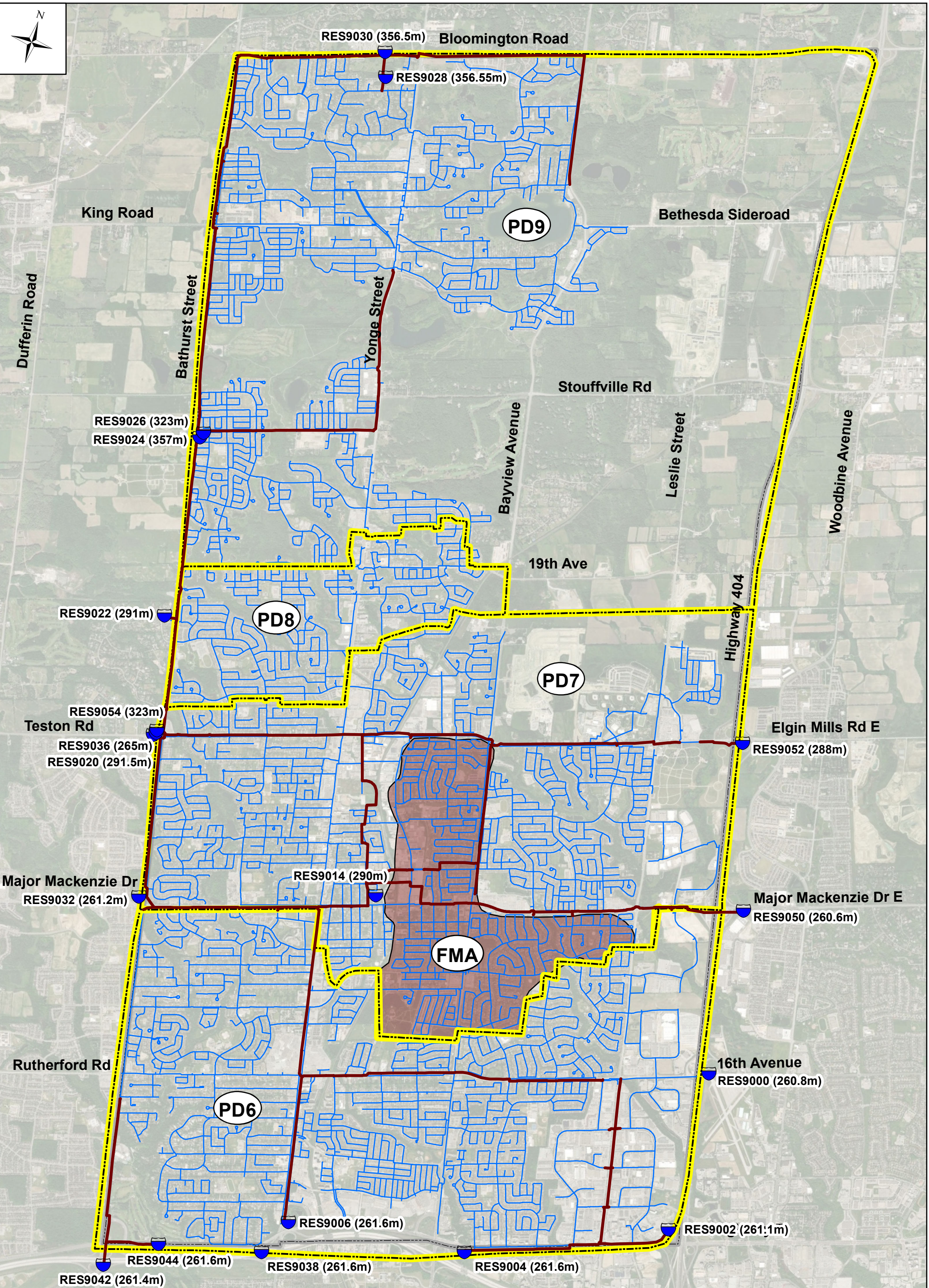
The City's water distribution system is a part of the York Region's system. The City provides water to its customers through four (4) Pressure Districts (PD6, PD7, PD8, and PD9, respectively). The existing conditions model reflects the system's condition to 2016. There were some changes in the system since 2016 that were only applied in the future conditions model. For example, the Flow Modulated Area (FMA) was removed in 2017. The valve setting in FMA was changed in 2017 which applied only to the future condition model. The 2016 pressure districts, including Flow Modulated Area (FMA), are displayed in **Figure 4-4**. Richmond Hill FMA was removed in 2017, which is discussed later in the report.

4.3.1 Water Model Development







The previous 2014 City-wide model was developed to simulate the water distribution system under 2011 conditions and was used to analyze the system until the Ultimate Build-Out. As a part of this study, Civica Infrastructure (Civica) updated the 2011 model employing the updated data provided by the City and York Region. The model is also verified in terms of assumptions and infrastructures and the required changes were made. New model scenarios are defined to simulate the water distribution network under 2041, 2051, and Ultimate Build-Out conditions.

The 2011 InfoWater model was developed as a steady state model; therefore, every connection to the Regional feed points, pumping stations, reservoir, and elevated tanks were modeled as fixed-head reservoirs. The head levels of the fixed-head reservoirs were established based on information obtained from the Region at the time. Valve settings (check valves, pressure reducing valves [PRVs], and flow control valves) were obtained from the City and the 2010 pressure monitoring report by Genivar. In terms of Hazen-William C factors, City's design values were assigned.

Civica examined the 2011 model assumptions and updated the model for 2016 conditions. Changes were made as required, employing information obtained from the City and Region. This included assumptions for model configuration, supply points, boundary conditions, watermains, pressure district boundaries, and valve settings.



Legend

-  York Region Reservoir Locations (19) (Head (m))
-  Local Watermains
-  Regional Watermains
-  (PD9) Pressure District Zones
-  FMA Area
-  Municipal Boundary

**Figure 4-4:
2016 Pressure Districts**

Drawn By: J.H. Date: Sep 27, 2023



4.3.1.1 Demand Allocation

Similar to the sanitary model, water demands were allocated to the model using the population and according to the City’s and York Region’s standards summarized in **Table 4-4**. Total water consumption during 2016 at each meter as well as total 2016 population (residential and ICI) for Traffic Zones was obtained from the City and Region. The 2016 water consumption for meters close to each demand node were calculated, and the population was assigned to the node proportional to the average water consumption. The Average Day, Max Day, and Peak Hour demands were then calculated for each demand node using the assigned populations (ICI and Residential) and agreed upon York Region and City’s design standards. For new developments, populations were assigned as described in **Appendix A**.

For the existing scenario (2016) population, the City’s water demand rate of 365 lpcd was used for both residential and ICI demands as per the City design criteria. For future population demand calculations, forecasted water demand rates from York Region’s 2016 Water and Wastewater Masterplan were adopted. Similarly, the regional Maximum day and peak hour demand factors were used for future demand calculations. Adopted water demand rates and peak factors are summarized in **Table 4-4**.

The City does not have a criterion for the minimum hour demand, therefore, the minimum rate factor of 0.8 was assumed as per MECP recommendation.

Table 4-4: City of Richmond Hill Standard Design Demand Factors

Year	Residential Demand (L/c/d)	ICI Demand (L/c/d)	Maximum Day Demand Factor	Peak Hour Demand Factor
2016	365	365	1.8	2.7
2041	189	144	1.8	2.7
2051	189	144	1.8	2.7
Ultimate Build-Out	189	144	1.8	2.7

City of Richmond Hill has no minimum rate recommendation. A minimum rate factor of 0.8 was used to calculate minimum hour demand in this project, as per MECP recommendation for communities with the population greater than 150,000

Boundary conditions of the system were verified and updated using Regional data. All points of supply to the distribution system were modeled as fixed-head reservoirs. The head levels of the fixed-head reservoirs were established based on information obtained from the Region.

4.3.2 Assessment of Existing Conditions

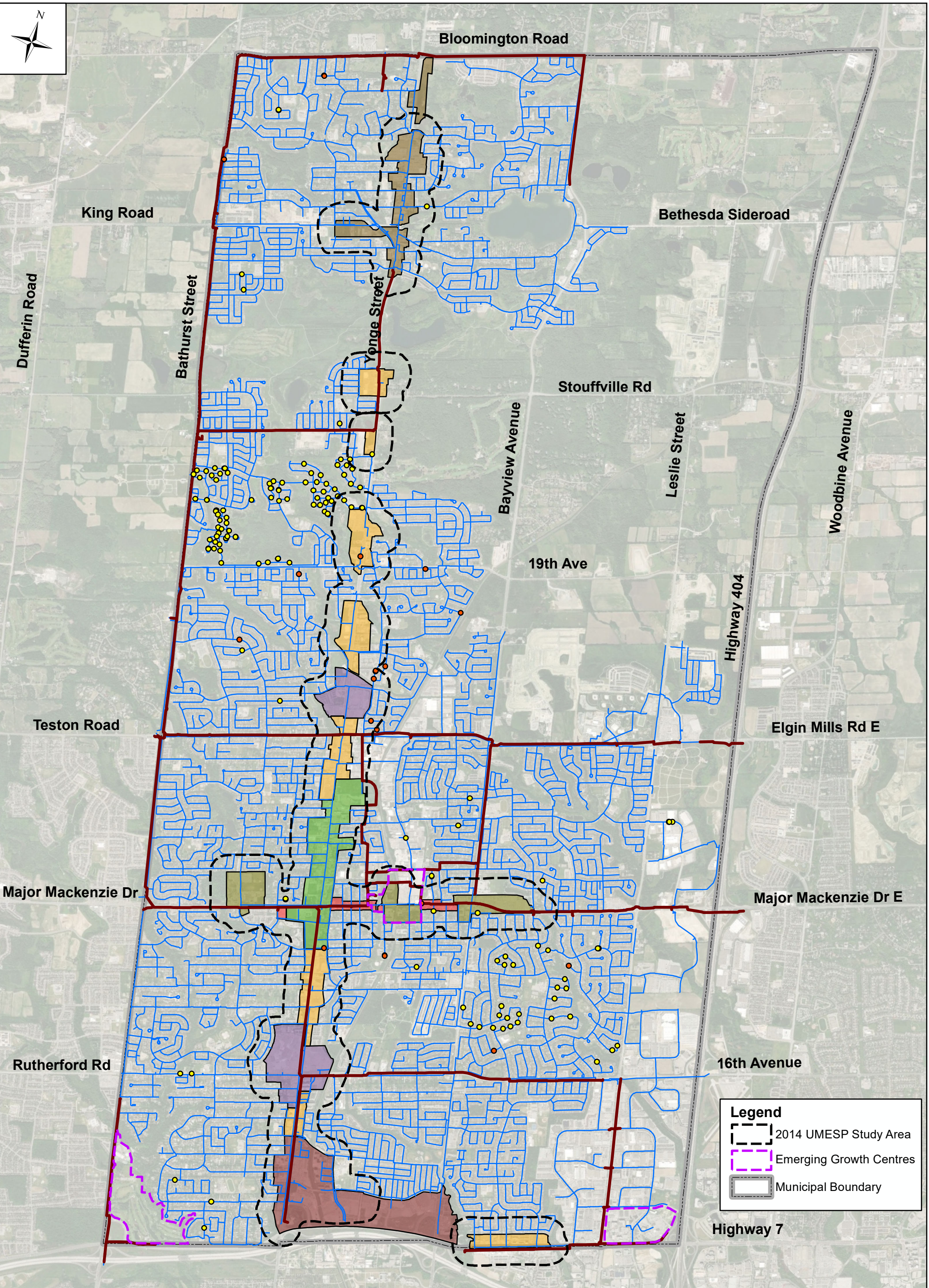
To verify the modeling results versus City static pressure test data, the model was run under 2016 Maximum Daily Demand and Peak Hour Demand. The difference between the model pressures and the 2016 observed pressures are shown in **Figure 4-5** and **Figure 4-6**. The modelling results are generally within 100 kPa of the hydrant test results. The areas with modeled pressures more than 100 KPa difference with the hydrant test data are:

- A. South of Major Mackenzie Drive and east of Bayview: this area is the south part of the old FMA. As mentioned before, FMA no longer exists and the area is now a part of PD7;
- B. South of Major Mackenzie Drive and east of Yonge Street;
- C. North of Gamble Street and west of Yonge Street. This area consists of sub-divisions in PD9 with five PRVs in the area. The PRV settings were not available at the time of this study, which affects

the modeling results. According to the City, the area has high-pressure fluctuations, which affects the hydrant test results.

Results outside of 100 KPa range tend to be clustered, which could indicate operational issues that aren't modelled correctly (partially-closed valves or possibly nearby watermains temporarily closed for operational/maintenance purposes), or hydrant test data that was not undertaken during the daily peak demand hour.

Further information about the water model development, calibration and results can be found in **Appendix B**.



Legend

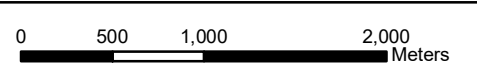
- 2014 UMESP Study Area
- Emerging Growth Centres
- Municipal Boundary

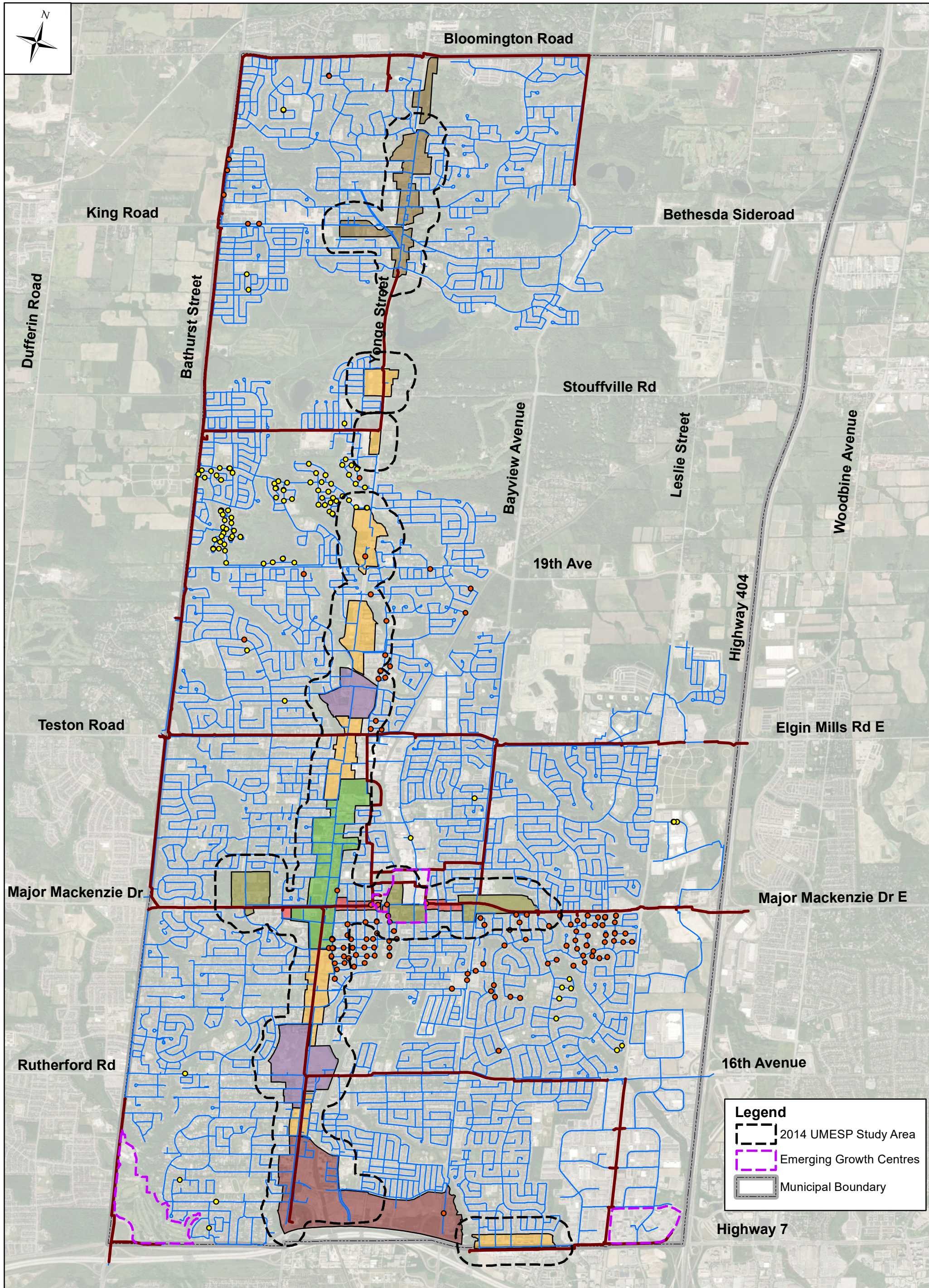
Legend

- Observed Pressure 100KPa Higher than Modeled
- Modeled Pressure 100KPa Higher than Observed
- Local Watermains
- Regional Watermains
- Village Local Centre
- Key Development Areas
- Local Development Areas
- Local Mixed Use Corridor
- Oak Ridges Local Centre
- Regional Mixed Use Corridors
- Richmond Hill Centre

Figure 4-5: 2016 Modeled Maximum Day Demand Pressures v.s. Observed Pressures

Drawn By: J.H. Date: Oct 28, 2023





Legend	
●	Observed Pressure 100KPa Higher than Modeled
●	Modeled Pressure 100KPa Higher than Observed
—	Local Watermains
—	Regional Watermains
■	Village Local Centre
■	Key Development Areas
■	Local Development Areas
■	Local Mixed Use Corridor
■	Oak Ridges Local Centre
■	Regional Mixed Use Corridors
■	Richmond Hill Centre

Figure 4-6:
Modeled Peak Hour Demand Pressures v.s. 2016 Observed Pressures

Drawn By: J.H. Date: Oct 28, 2023



5.0 Future Conditions & Design Criteria

5.1 Future Development Scenarios

This section summarizes the assessment of the water and wastewater sewer systems considering the following incremental growth scenarios: 2041, 2051 and Ultimate build-out conditions. Incremental development conditions assume that each successive scenario adds flows derived from population growth from the previous scenarios as per the expected development sequence.

The areas and populations added in each future scenario have been defined in consultation with the City and from the best available planning documents, e.g. Traffic Zone Projections and Development Applications. Initially, the future population was estimated and distributed using the Traffic Zone Projections. These estimations were further refined based on known development applications. This means that the estimated population from known development applications was subtracted from the projected Traffic Zone population. For example, if the projected 2041 population for a given TZ is 10,000 people, and in that TZ there are some development applications with an estimated population of 2,000 people, then, it is assumed that the final projected TZ population to be distributed within the study area is 8,000 people.

A breakdown of the estimated future population for each growth scenario is presented in **Table 5-1** below.

Table 5-1: Population Breakdown Per Growth Scenario

Description	Growth Scenario		
	2016 2041	2041 2051	2051 Ultimate Build out
Residential population ¹	107,192	18,905	121,442
Employment population ²	30,571	12,144	49,580
Other residential and employment population ³			39,122
Total Population Added	137,763	31,049	210,144

NOTES:

- 1) Includes population from active development applications. Also, in TZ where population decreased, the 2016 population was kept.
- 2) In TZ where employment decreased, the 2016 employment numbers were kept.
- 3) Includes additional population growth from Richmond Hill Centre Secondary Plan, OPAs for Oak Ridges Local Centre and the Village Core, beyond original 2021 forecast.

5.2 Future Wastewater System Conditions

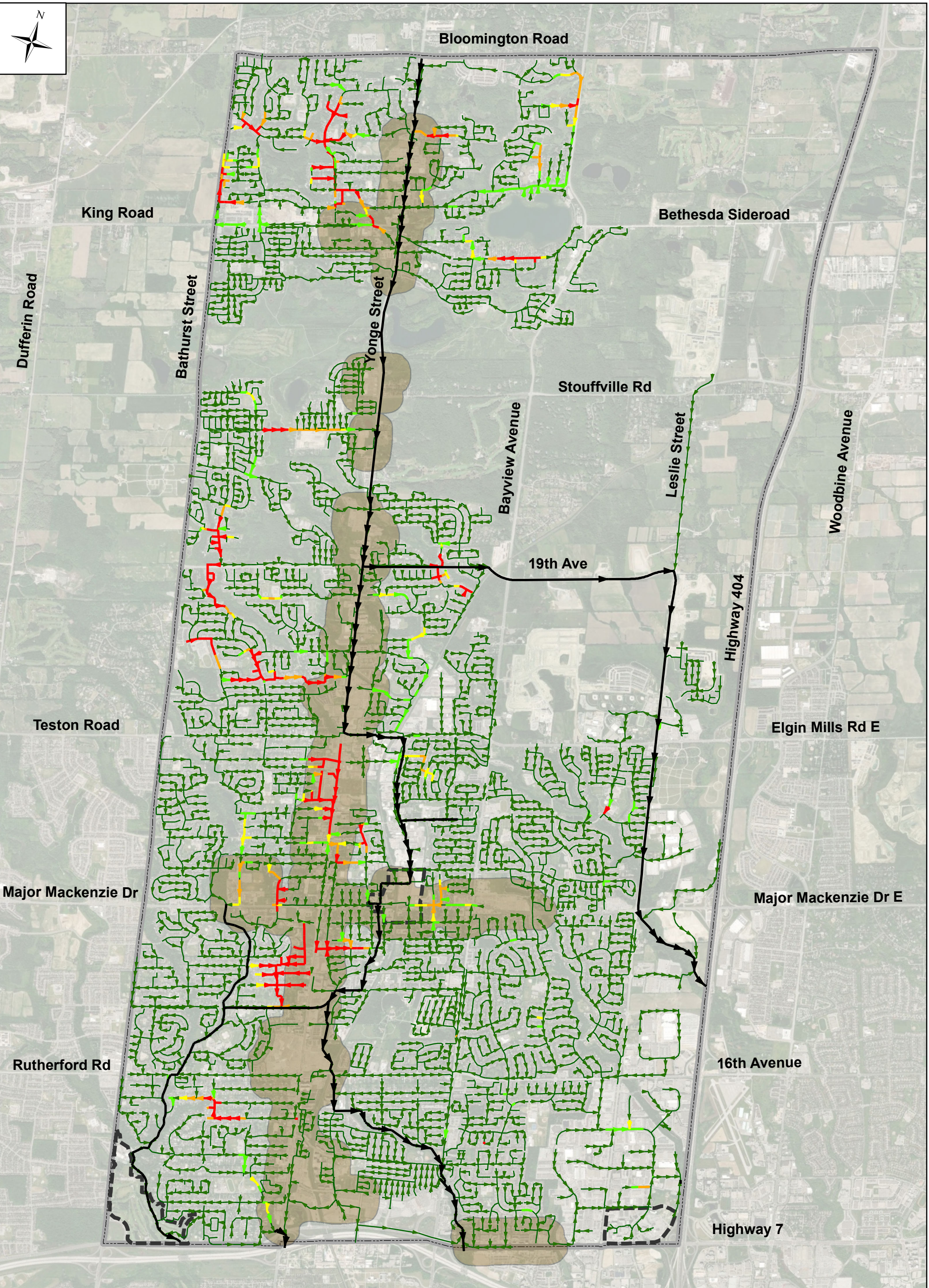
The calibrated model was used to assess the sanitary system under extreme wet-weather conditions, specifically, during the City's 1 in 25-year design storm. The storm is a six-hour event with a time-to-peak ratio of 0.33 (Chicago-type storm), with a peak intensity of 172 mm/hr over 10-min intervals.

The per capita flow rate assigned for areas without flow monitoring data and for future population was 265 L/c/d, and the infiltration allowance was assumed to be 0.26 L/s/ha.

In terms of level of service, an agreement was reached with the City that sewer upgrades should be generally implemented when the surcharge level exceeds 0.3 meters above the obvert during the 25-year design storm. The evaluation of surcharge levels and acceptance was conducted on a case-by-case basis.

As mentioned above, if the system surcharges during Existing Conditions, i.e. the surcharge was not triggered by future growth within the Study Area, then remedial measures will not be evaluated as part of this study. Further studies are needed to evaluate proposed solutions and eliminate surcharge conditions in locations outside of the study area, and surcharge caused by existing developments (2016 population).

The sewer performance under each growth scenario is depicted in **Figure 5-1** to **Figure 5-3**. As expected, the level of surcharge increases from the 2041 to the Ultimate Build-out growth scenario.



N

Dufferin Road

King Road

Bathurst Street

Yonge Street

Bloomington Road

Bethesda Sideroad

Stouffville Rd

Bayview Avenue

19th Ave

Leslie Street

Woodbine Avenue

Teston Road

Major Mackenzie Dr

Rutherford Rd

Highway 404

Elgin Mills Rd E






Major Mackenzie Dr E

16th Avenue

Highway 7

Legend

Manhole Water Depth (m) (Water Level to Obvert)

-  No Surcharge
-  0 - 0.15
-  0.15 - 0.3
-  0.3 - 0.6
-  > 0.6





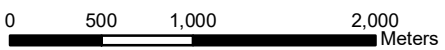
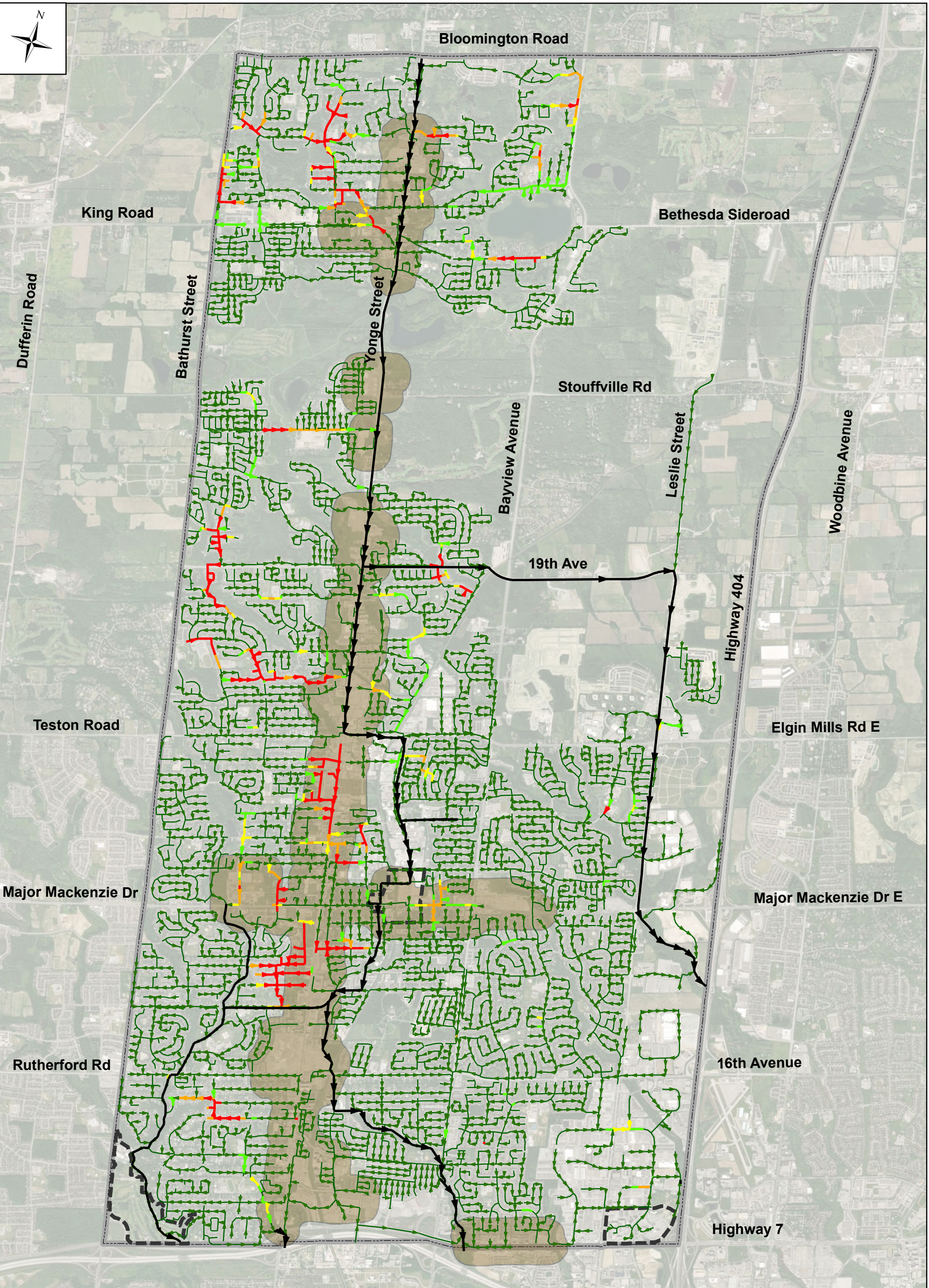
-  York Region Sanitary Trunk Sewers
-  Study Area Boundary
-  Emerging Growth Centres
-  Municipal Boundary

Figure 5-1: Level of Surcharge Under 2041 Conditions

Drawn By: J.H. Date: Oct 28, 2023

0 500 1,000 2,000 Meters





Legend

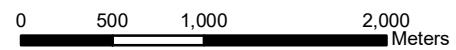
Manhole Water Depth (m)
(Water Level to Obvert)

- No Surcharge
- 0 - 0.15
- 0.15 - 0.3
- 0.3 - 0.6
- > 0.6

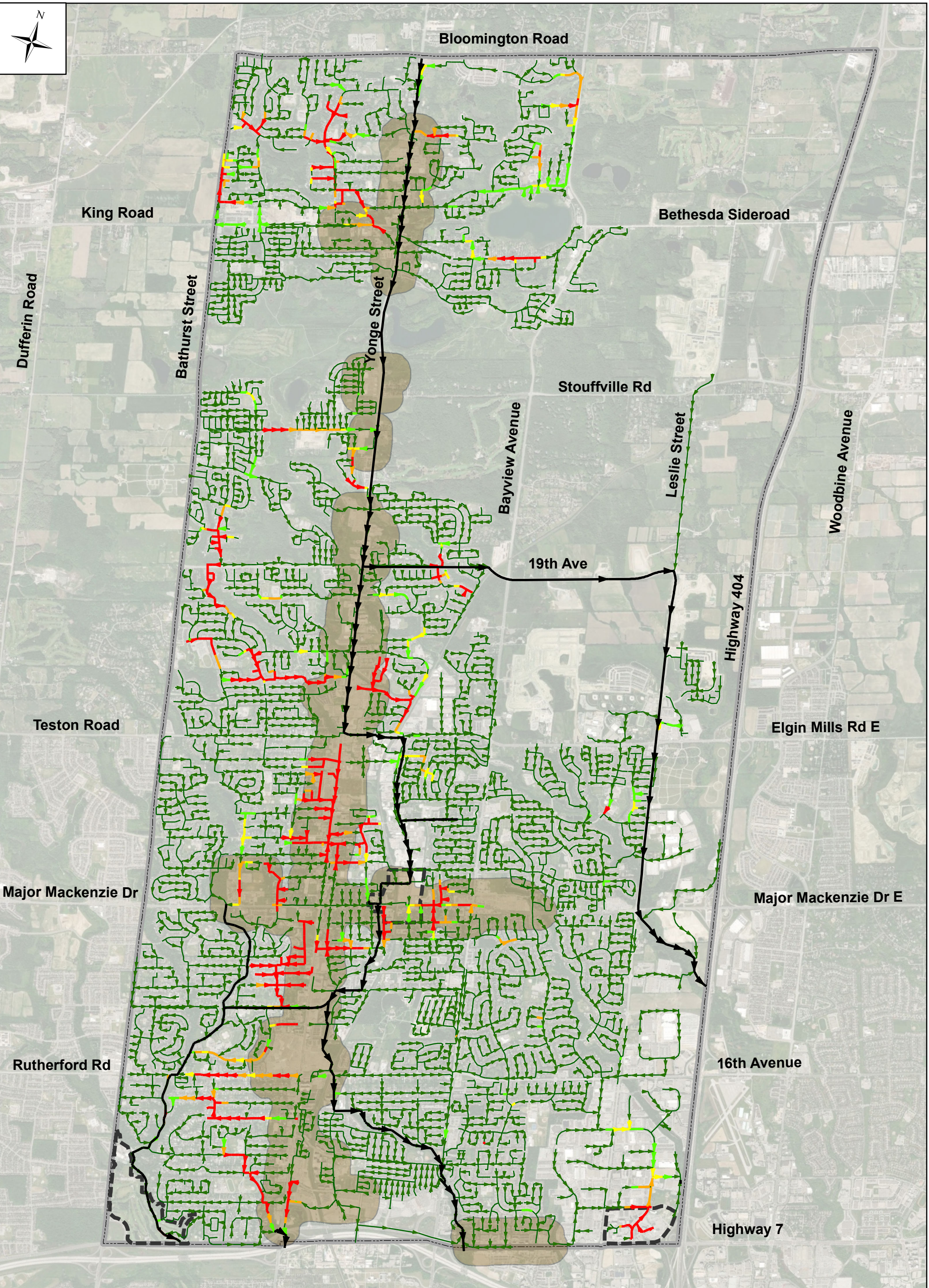
- York Region Sanitary Trunk Sewers
- ▭ Study Area Boundary
- ▭ Emerging Growth Centres
- ▭ Municipal Boundary

**Figure 5-2:
Level of Surcharge Under
2051 Conditions**

Drawn By: J.H. Date: Oct 28, 2023



**RIC18-0004 -
Richmond Hill UMESP Update**



Legend

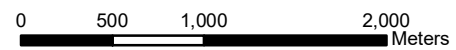
**Manhole Water Depth (m)
(Water Level to Obvert)**

- No Surcharge
- 0 - 0.15
- 0.15 - 0.3
- 0.3 - 0.6
- > 0.6

- York Region Sanitary Trunk Sewers
- ▭ Study Area Boundary
- ▭ Emerging Growth Centres
- ▭ Municipal Boundary

**Figure 5-3:
Level of Surcharge Under
Ultimate Build-Out**

Drawn By: J.H. Date: Oct 28, 2023



**RIC18-0004 -
Richmond Hill UMESP Update**

5.3 Future Water System Conditions

To review the need for infrastructure upgrades within the study areas, the InfoWater model was run under the following scenarios:

- Ultimate Build-Out Peak Hour Demand: the model was run under this scenario to examine if the future demands can be supplied while maintaining the minimum pressure of 275 KPa;
- 2041 Minimum Hour Demand: the model was run under the 2041 minimum day demand to examine if the future demands can be supplied while maintaining the maximum pressure of 690 Kpa; and,
- Fire Flow Scenario: the model was run under Ultimate Build-Out Maximum Day Demand plus fire flow demands to investigate the available fire flow at each node while maintaining a minimum pressure of 140 KPa in demand nodes.

As discussed, the model was validated with 2016 observed data and valve setting; however, the valve setting in FMA that changed in 2017 was applied to the future condition model. Furthermore, the transmission main at Bathurst and Major Mackenzie will be recommissioned by the City before 2031 and feed the system in the future conditions. The future model also includes the new water mains on the BRT line located on Yonge Street, south of Major Mackenzie Drive. **Figure 5-4** displays the updated Pressure districts after removing the FMA.

As per the previous UMESP, the modeling results within the 100 KPa of the observed pressures were assumed acceptable. In terms of Static Pressure, the acceptable criteria was set as follows:

- Minimum pressure during the peak hourly demand, 275 Kpa.
- Maximum pressure during average day demand 690 Kpa.

For fire flow, the minimum pressure at all demand nodes should be more than 140 KPa when the system is tested for fire flow during maximum day demand conditions. It is worth noting that Richmond Hill design criteria indicate that the City's minimum pressure during fire flow plus maximum day demand should be 275 KPa. After communications with the City, the minimum pressure of 140 KPa was used. The later value (140 KPa) was also used in the previous UMESP.

For the purposes of this UMESP, fire flow demands of 317 l/s are recommended for high rises. Required fire flow demand was recommended for each land use designation by reviewing the City's land use policy explained in Richmond Hill Official Plan, and after consultation with the City. **Table 5-2** summarizes the recommended fire flow demands used in this study.

Note that the purpose of this study was to investigate water network conditions within the study area. It is proposed that the City conducts studies to investigate the effect of the new developments on conditions outside of the study area. For example, the area south of Lake Wilcox and some areas north of Teston Road would not meet the City's criteria under future conditions.

Table 5-2: Recommended Fire Flow Demand for Land Use Designations (APPENDIX B: Final Urban MESP Update Study Water Servicing and Results)

Structure Type	FF Demand (L/s)
Single Family	67
Townhouses and Apartments up to 3 stories	100
Institutional, Industrial, Commercial	190
Apartments up to 8 stories	190
Medium to High-Rise buildings	317
Big box Retail	317

5.3.1 Minimum Pressure During the Peak Hourly Demand

The minimum pressure was investigated under Ultimate Build-Out, Peak Hour Demand. Results are shown in **Figure 5-5**. All nodes within the study area satisfy the minimum pressure requirement (275 KPa), except for a small area in Downtown Local Center.

5.3.2 Maximum Pressure During the Minimum Hourly Demand

The maximum pressure was investigated under 2041 Minimum Hour Demand. As shown in **Figure 5-6**, there are two areas with pressures higher than 690 KPa inside or close to the study area:

- Yonge Street north of Elgin Mills, and Canyon Hill Avenue west of Yonge and Elgin Mills Key Development Area
- Church Street, south of Major Mackenzie Dr and east of Yonge Street

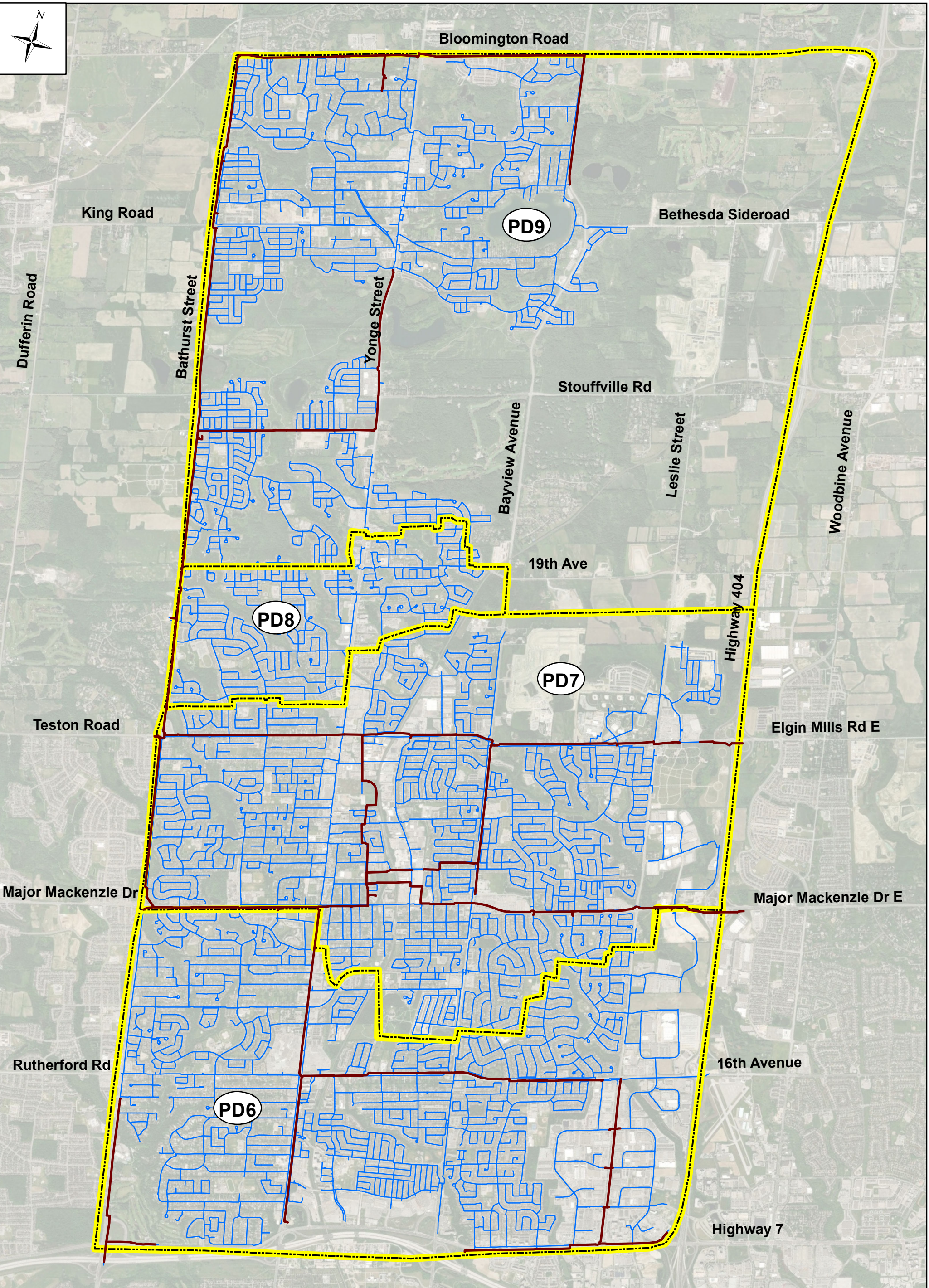
As high pressures exceeding 690 kPa are not triggered by population growth, solutions for these areas will not be recommended in this study.

5.3.3 Fire Flow Demand

Available fire flow required for each land use and development application was investigated by considering the fire flow demands discussed in previous sections under the Ultimate Build-Out maximum day demand. **Appendix B** shows the required and available fire flows (for a minimum pressure of 140 kPa) for every OP subdivision, Emerging Growth Centre and Development application in the study area. Major flow deficiencies were observed in the following areas:

- Oak Ridges Local Center at Yonge Street and Elm Grove Avenue. The existing watermains cannot support the institutional and commercial growth in the area.
- Yonge Street at Stouffville Road and Murihead Crescent. There are no existing municipal watermains to service growth east of Yonge Street.
- Yonge Street North of Gamble Road. There are not existing municipal watermains to service growth west of Yonge Street.
- Yonge Street East Side, North of Silverwood Ave. There are not any existing municipal watermains to service growth east of Yonge Street.
- Yonge Street & Oxford Street. The existing watermains cannot support the growth west of Yonge Street.

- Church Street. Existing 150mm watermain cannot support the growth west of Church Street between Lorne Avenue and Dunbarton Court.
- Yonge Street between Major Mackenzie Drive East and Elmwood Avenue. The existing 150mm watermain cannot support the growth east of Yonge Street.
- 150 mm watermains on Cedar Avenue, Norfolk Avenue and Old Markham Road cannot support growth in these areas.
- Yonge Street at Yonghurst Drive and Clarissa Drive. Existing watermains cannot support growth or not available to service future intensification areas east side of Yonge Street.
- Spruce Avenue, Oak Avenue, Edgar Avenue, Scott Drive, Mackay Drive and Garden Avenue at intersections with Yonge Street. Existing 150 mm watermains cannot support growth in these areas.
- Richmond Hill Centre: High Tech Road between Yonge St and Red Maple Rd. Existing 150 mm watermains cannot support growth in the area.



Dufferin Road

King Road

Bloomington Road

PD9

Bethesda Sideroad

Bathurst Street

Yonge Street

Stouffville Rd

Bayview Avenue

Leslie Street

Woodbine Avenue

19th Ave

PD8

PD7

Highway 404

Teston Road

Elgin Mills Rd E

Major Mackenzie Dr

Major Mackenzie Dr E

Rutherford Rd

PD6

16th Avenue

Highway 7



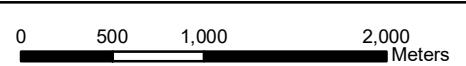
RIC18-0004 - Richmond Hill UMESP Update

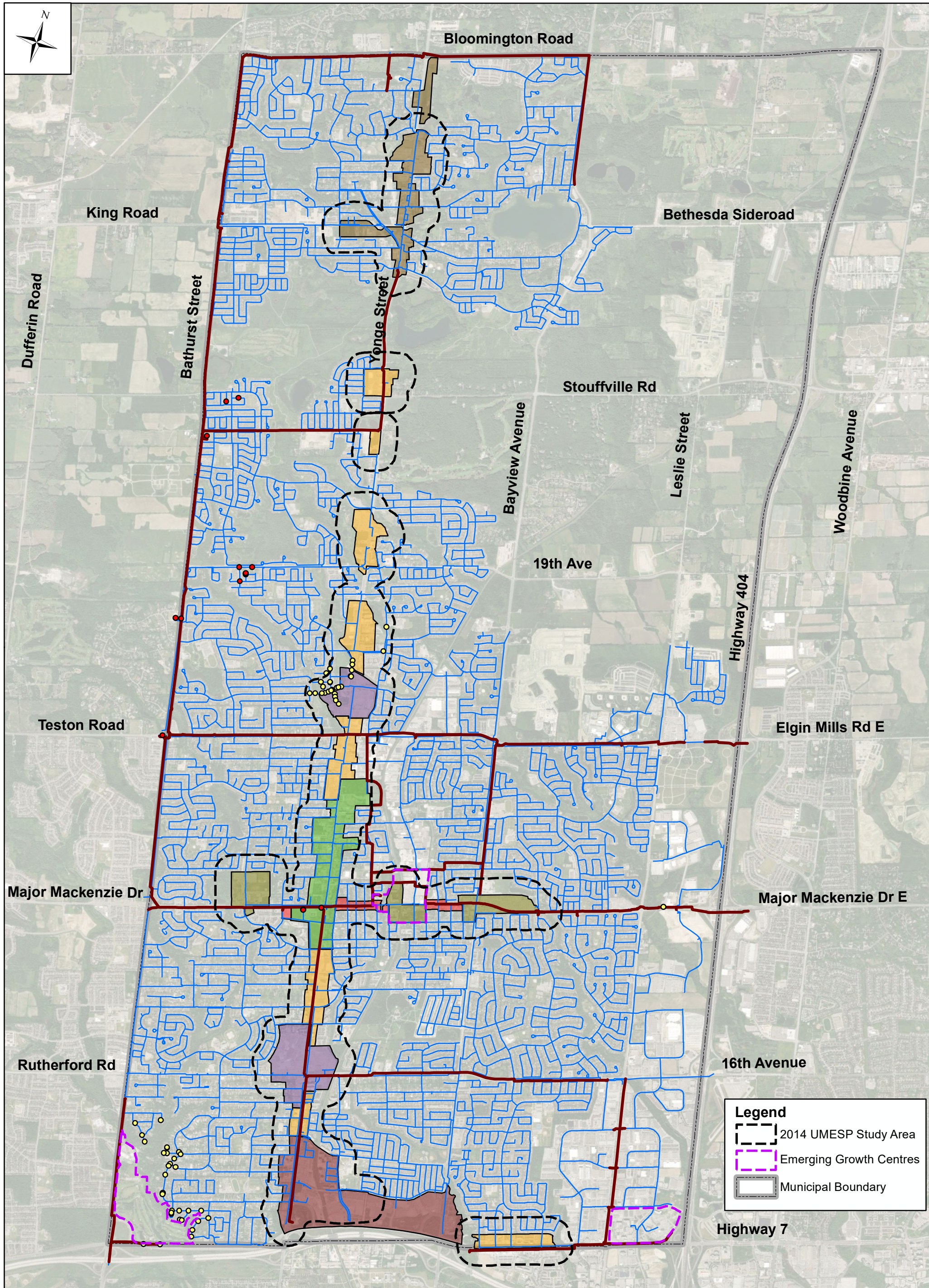
Legend

- (PD9) Pressure District Zones
- Municipal Boundary
- Local Watermains
- Regional Watermains

**Figure 5-4:
Updated Pressure Districts After
Removing FMA (2017)**

Drawn By: J.H. Date: Sep 27, 2023





Peak Hour Demand Pressure (KPa)

- < 275
- > 690

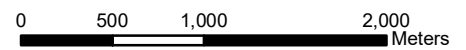
Note: Nodes with normal pressure (275 - 690 KPa) are not shown

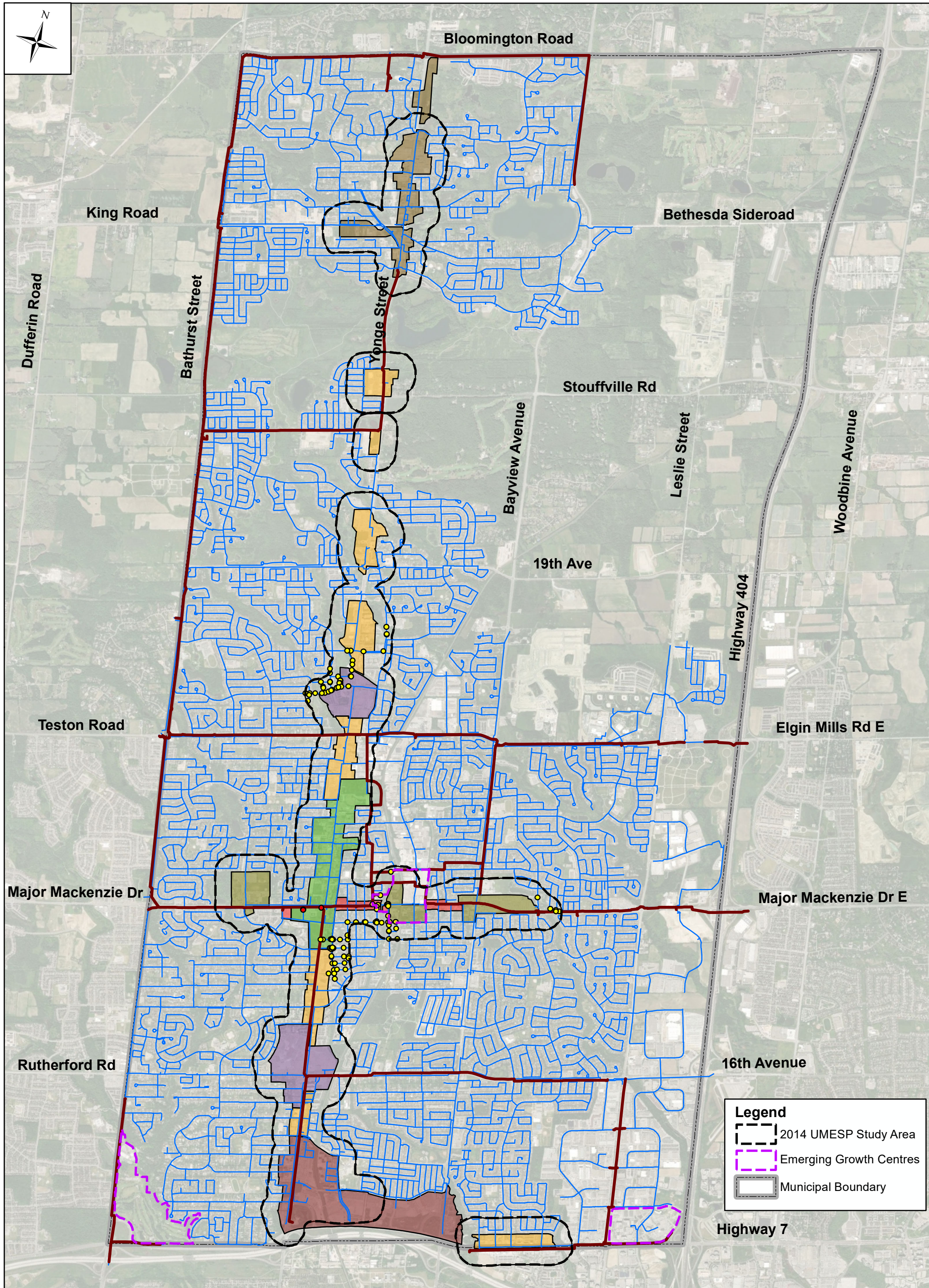
- Local Watermains
- Regional Watermains

- Village Local Centre
- Key Development Areas
- Local Development Areas
- Local Mixed Use Corridor
- Oak Ridges Local Centre
- Regional Mixed Use Corridors
- Richmond Hill Centre

**Figure 5-5:
Peak Hour Demand Pressures
under Ultimate Built-Out Condition**

Drawn By: J.H. Date: Oct 28, 2023





Minimum Hour Demand Pressure (KPa)

- < 275
- > 690

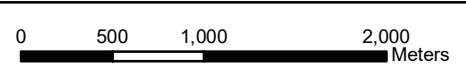
Note: Nodes with normal pressure (275 - 690 KPa) are not shown

- Local Watermains
- Regional Watermains

- Village Local Centre
- Key Development Areas
- Local Development Areas
- Local Mixed Use Corridor
- Oak Ridges Local Centre
- Regional Mixed Use Corridors
- Richmond Hill Centre

**Figure 5-6:
Minimum Hour Demand Pressures Under 2041 Conditions (Inside the Study Area)**

Drawn By: J.H. Date: Oct 28, 2023



6.0 Development & Evaluation of Alternatives

To minimize the risk of the risk of inadequate level of service triggered by the ultimate Build-out population, four (4) alternative solutions were evaluated. These alternative solutions are described in **Table 6-1** below.

Table 6-1: Evaluation of Wastewater Alternatives

Alternative	Description	Results of Evaluation
1. Do Nothing	A do-nothing scenario would allow Build-out of the City structure in accordance with the Official Plan, without making improvements to the sanitary collection system.	This approach would result in a substandard level of service through some parts of the City, with the potential for sanitary sewer surcharging as build-out occurs. This could then trigger basement flooding, and possibly even surface flooding. This alternative does not satisfy the objectives associated with the problem statement for this project but provides an important baseline for comparison to other viable solutions.
2. Limit Community Growth	This alternative would require updating the Official Plan in order to limit the extent of intensification to only what could be accommodated within the existing sanitary sewers. In order to accommodate the provincially-mandated growth, the City would have to designate additional areas within the City for intensification, where sufficient reserve sanitary servicing capacity exists.	This alternative solution would ultimately negate some of the planning processes that have been undertaken to date, and therefore be unsatisfactory with respect to the objectives associated with the problem statement for this project.
3. Implement Water Conservation Procedures	This alternative solution would effectively increase the population that could be serviced through the existing collection system by decreasing the average per-capita flow rates across the City.	While many municipalities have successfully reduced the overall flow generation rate through implementing education and incentive-based water conservation procedures, the success of these programs in general – and within the specific areas where a flow reduction would be required in order to service the planned growth – is not guaranteed.
4. Enhance / Expand the Existing Collection System	This alternative would have the City plan for strategic enhancements to and/or expansion of the existing collection system in order to accommodate the planned populations at the prescribed level of service in accordance with the City’s design criteria.	The future condition model was used to identify the extent of system upgrades needed to compensate for any observed shortfalls.

Alternatives 1 and 2 are not recommended within the context of the problem statement for this project. Alternative 3 is not recommended because the flow reduction that would be required to service the planned future growth is not guaranteed. Therefore, Alternative 4 is the preferred solution for this Study.

6.1 Wastewater Solutions

Based on the evaluation of the four (4) alternatives, Alternative 4 was selected as the preferred solution. The mitigation measures proposed as part of Alternative 4 are listed and described in **Table 6-2**. The overall preferred remedial measures for the Study Area can be seen in **Figure 6-1** and the detailed maps for every proposed project are provided in **Appendix A**.

As mentioned above, sanitary sewers that are surcharging but were not triggered by future growth within the study area will not be evaluated for remedial measures during this study. Further studies are recommended to address existing capacity constraints throughout the City. These locations are highlighted in blue in **Figure 6-1**.

For the purposes of this report, all recommended pipe replacements refer to replacing an existing pipe with a larger sized pipe (i.e. upsizing/upgrading). Although, it has been assumed that most of the proposed solution will be located within the City's right-of-way, some of the crossings, new pipes, and new connections and will be affecting the Regional infrastructure.

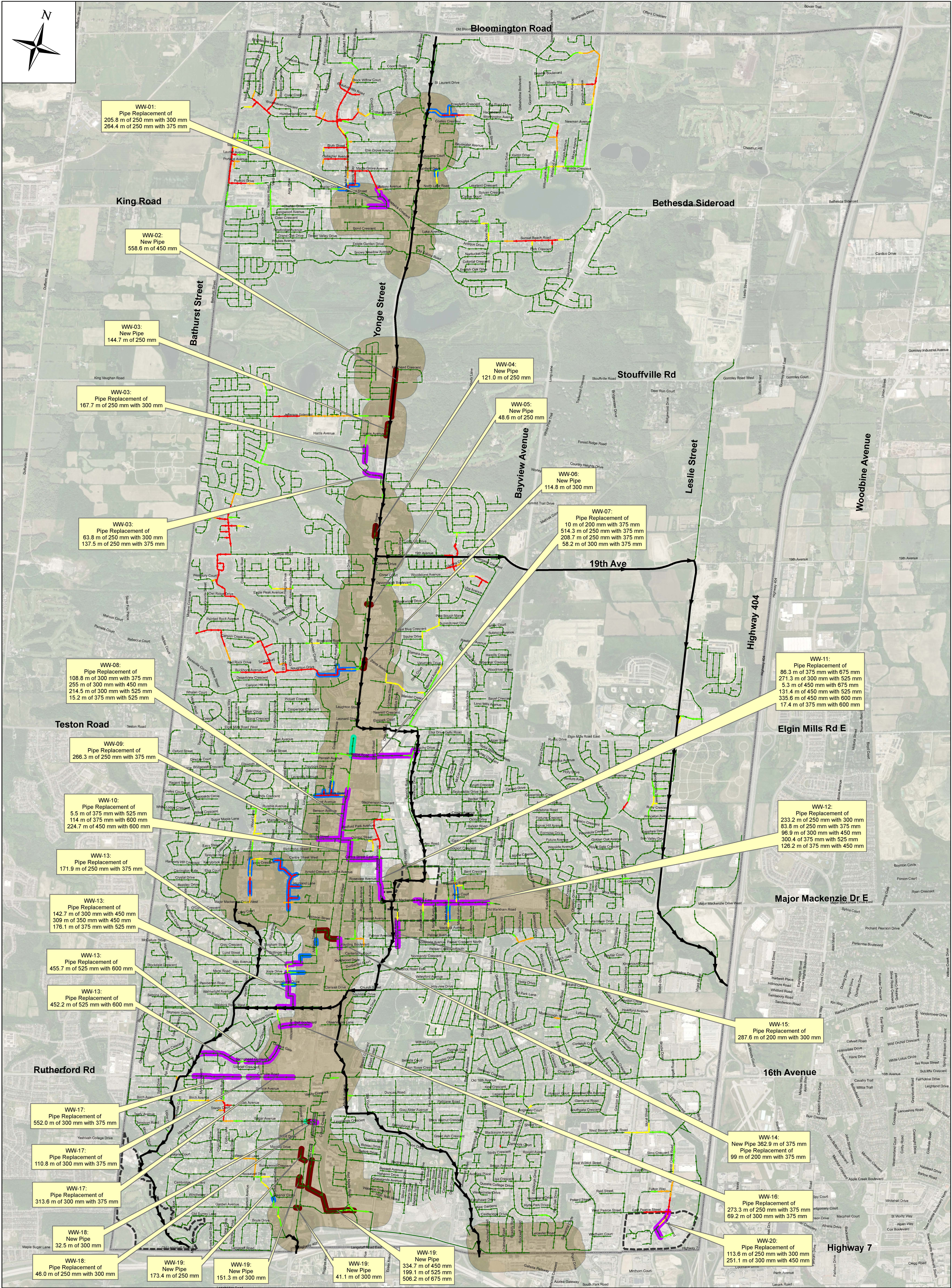
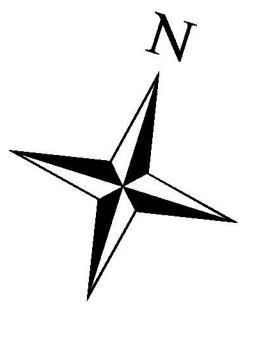
Table 6-2: Recommended Wastewater Projects

Project	2014 UMESP Project ID	Description	Location	From	To	Existing Diameter (mm)	Recommended Diameter (mm)	Length (m)
WW-1	n/a	Sewer Upgrade	King Road and Schomberg Road	on King Street	on Schomberg Road	250	300 and 375	470.2
WW-2	WW-17	New Pipe	Yonge Street	Muirhead Crescent	Jefferson Sideroad	NA	450	558.6
WW-3	WW-16	New Pipe & Sewer Upgrade	Yonge Street, Grange Drive and Townwood Drive	on Yonge Street	on Townwood Drive	250	250, 300 and 375	513.7
WW-4	n/a	New Pipe	Easement along Yonge Street	NA	NA	NA	250	121
WW-5	n/a	New Pipe	Yonge Street	NA	NA	NA	250	48.6
WW-6	WW-15	New Pipe	Yonge Street and easement	NA	NA	NA	300	114.8
WW-7	WW-13, WW-14	Sewer Upgrade	Industrial Road, Newkirk Road and Beechy Drive	Yonge Street / Industrial Road	Beechy Drive / Newkirk Road	300, 250 and 200	375	791.2
WW-8	WW-11	Sewer Upgrade	Yonge Street	Levendale Road	Dunlop Street	300 and 375	375, 450 and 525	593.5
WW-9	WW-12	Sewer Upgrade	Wright Street	Hall Street	Yonge Street	250	375	266.3
WW-10	WW-11	Sewer Upgrade	Dunlop Street and Church Street	Yonge Street	Centre Street East	375 and 450	525 and 600	344.2
WW-11	WW-11	Sewer Upgrade	Centre Street East and Pugsley Avenue	Church Street	Major Mackenzie Drive East	300, 375 and 450	525, 600 and 675	847.3

Project	2014 UMESP Project ID	Description	Location	From	To	Existing Diameter (mm)	Recommended Diameter (mm)	Length (m)
WW-12	n/a	Sewer Upgrade	Major Mackenzie Drive East and Easement	Bayview Avenue	Essex Avenue	250, 300 and 375	300, 375, 450 and 525	840.5
WW-13	WW-10	Sewer Upgrade	Addison Street and Easement, May Avenue Easements, Weldrick Road West and Springhead Gardens	Addison Street	Kitsilano Crescent	250, 300, 350, 375 and 525	375, 450, 525 and 600	1707.6
WW-14	WW-9	Sewer Upgrade, and New Pipe	Addison Street, Yonge Street Easement, City Park, Church Street South	Addison Street	Harding Boulevard	200	375	461.9
WW-15	n/a	Sewer Upgrade	Cedar Avenue	Elmwood Avenue	South of Palmer Avenue	200	300	287.6
WW-16	n/a	Sewer Upgrade	Baif Boulevard	Yonge Street	Springhead Gardens	250 and 300	375	342.5
WW-17	n/a	Sewer Upgrade	Carrville Road	Yonge Street	Duncombe Lane	300	375	976.4
WW-18	WW-04	Sewer Upgrade, and New Pipe	Dalemout Gate	Yonge Street	Ellesmere Street	250	300	78.5
WW-19	n/a	New Pipe	Yonge Street, High Tech Road, RHC Street, Easements and Red Maple Road	Yonge Street	Red Maple Road	NA	250, 300, 450, 525 and 675	1405.8
WW-20	n/a	Sewer Upgrade	York Boulevard, East Beaver Creek Road	York Boulevard	Norman Bethune Avenue	250 and 300	300 and 450	364.7

Notes:

1. Sewer Upgrades refer to replacing an existing pipe with a larger sized pipe.
2. All proposed works are assumed to be within the City's right-of-way.



WW-01:
Pipe Replacement of
205.8 m of 250 mm with 300 mm
264.4 m of 250 mm with 375 mm

WW-02:
New Pipe
558.6 m of 450 mm

WW-03:
New Pipe
144.7 m of 250 mm

WW-03:
Pipe Replacement of
167.7 m of 250 mm with 300 mm

WW-04:
New Pipe
121.0 m of 250 mm

WW-05:
New Pipe
48.6 m of 250 mm

WW-06:
New Pipe
114.8 m of 300 mm

WW-07:
Pipe Replacement of
10 m of 200 mm with 375 mm
514.3 m of 250 mm with 375 mm
208.7 m of 250 mm with 375 mm
58.2 m of 300 mm with 375 mm

WW-03:
Pipe Replacement of
63.8 m of 250 mm with 300 mm
137.5 m of 250 mm with 375 mm

WW-08:
Pipe Replacement of
108.8 m of 300 mm with 375 mm
255 m of 300 mm with 450 mm
214.5 m of 300 mm with 525 mm
15.2 m of 375 mm with 525 mm

WW-11:
Pipe Replacement of
86.3 m of 375 mm with 675 mm
271.3 m of 300 mm with 525 mm
5.3 m of 450 mm with 675 mm
131.4 m of 450 mm with 525 mm
335.6 m of 450 mm with 600 mm
17.4 m of 375 mm with 600 mm

WW-09:
Pipe Replacement of
286.3 m of 250 mm with 375 mm

WW-10:
Pipe Replacement of
5.5 m of 375 mm with 525 mm
114 m of 375 mm with 600 mm
224.7 m of 450 mm with 600 mm

WW-12:
Pipe Replacement of
233.2 m of 250 mm with 300 mm
83.8 m of 250 mm with 375 mm
96.9 m of 300 mm with 450 mm
300.4 of 375 mm with 525 mm
126.2 m of 375 mm with 450 mm

WW-13:
Pipe Replacement of
171.9 m of 250 mm with 375 mm

WW-13:
Pipe Replacement of
142.7 m of 300 mm with 450 mm
309 m of 350 mm with 450 mm
176.1 m of 375 mm with 525 mm

WW-13:
Pipe Replacement of
455.7 m of 525 mm with 600 mm

WW-13:
Pipe Replacement of
452.2 m of 525 mm with 600 mm

WW-15:
Pipe Replacement of
287.6 m of 200 mm with 300 mm

WW-17:
Pipe Replacement of
552.0 m of 300 mm with 375 mm

WW-17:
Pipe Replacement of
110.8 m of 300 mm with 375 mm

WW-17:
Pipe Replacement of
313.6 m of 300 mm with 375 mm

WW-18:
New Pipe
32.5 m of 300 mm

WW-18:
Pipe Replacement of
46.0 m of 250 mm with 300 mm

WW-19:
New Pipe
173.4 m of 250 mm

WW-19:
New Pipes
151.3 m of 300 mm

WW-19:
New Pipe
41.1 m of 300 mm

WW-19:
New Pipe
334.7 m of 450 mm
199.1 m of 525 mm
506.2 m of 675 mm

WW-14:
New Pipe 362.9 m of 375 mm
Pipe Replacement of
99 m of 200 mm with 375 mm

WW-16:
Pipe Replacement of
273.3 m of 250 mm with 375 mm
69.2 m of 300 mm with 375 mm

WW-20:
Pipe Replacement of
113.6 m of 250 mm with 300 mm
251.1 m of 300 mm with 450 mm

Legend

Recommended Solutions:

- Pipe Replacement
- New Pipe
- Sewer Installed through Viva Next BRT

**Manhole Water Depth (m)
(Water Level to Obvert)**

- No Surge
- 0 - 0.15
- 0.15 - 0.3
- 0.3 - 0.6
- > 0.6

- Sewer Surcharging During Existing Conditions
- York Region Sanitary Trunk Sewers
- Study Areas
- Emerging Growth Centres
- Municipal Boundary

**Figure 6-1:
Recommended Wastewater Projects**

Drawn By: J.H. Date: Oct 28, 2023



**RIC18-0004 -
Richmond Hill UMESP Update**

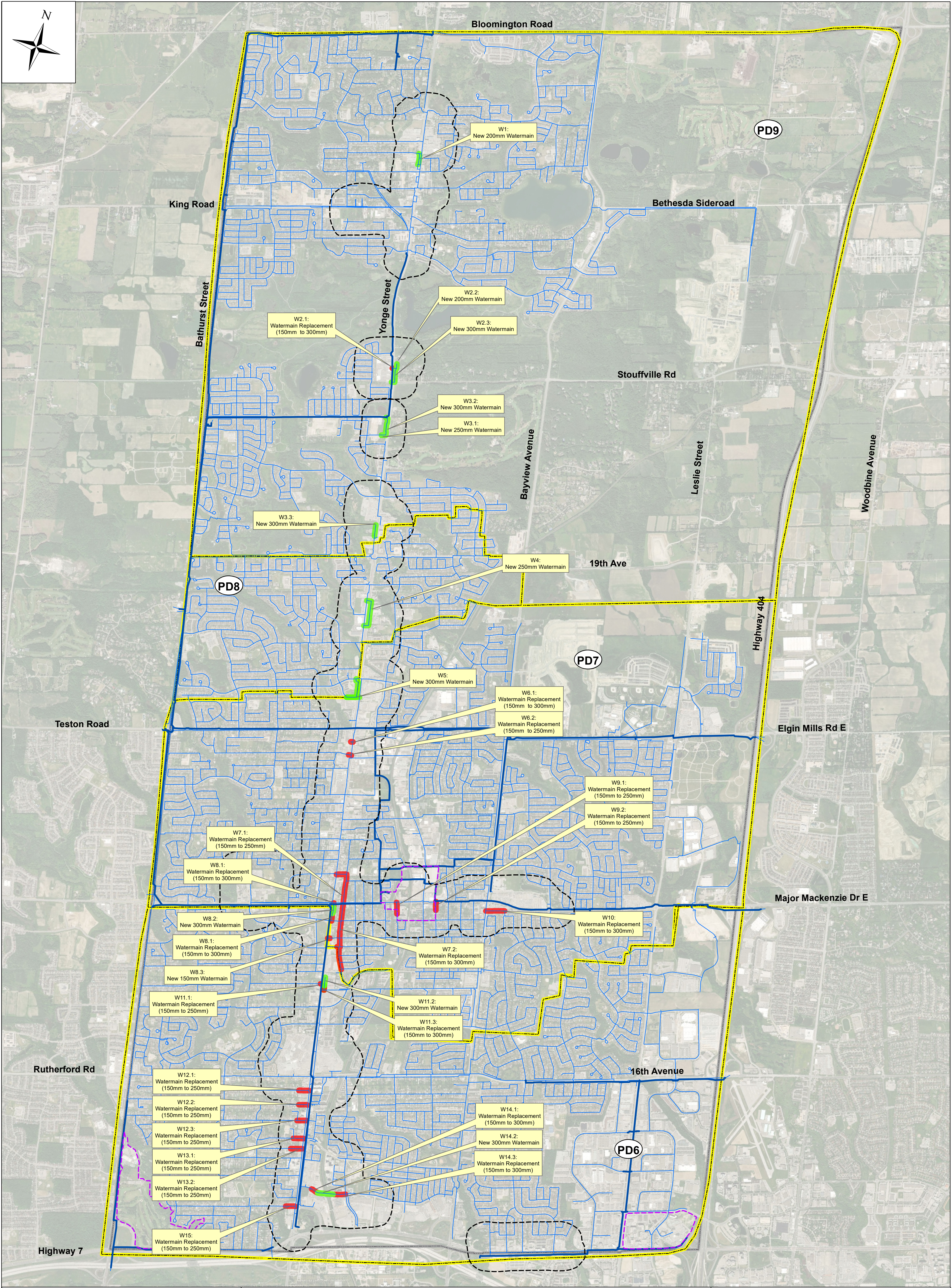
6.2 Water Solutions

To identify the required solutions, Civica initially investigated the deficiencies in the study area and reported the possible deficiencies to the City. The recognized areas were reviewed by the City and the final solutions were proposed for the locations where upgrades deemed necessary by the City. **Figure 6-2** illustrates a schematic of the required changes in the water network and **Table 6-3** summarizes the proposed projects. Figures showing the details of each project are presented in **Appendix B**.

Table 6-3. Proposed Projects - Water Distribution System

Project ID	Description	OP Area	Location	Year
W1	New 150 mm Watermain	Oak Ridges Local Centre	Yonge Street South of Elm Grove Avenue	2041
W2.1	Watermain Replacement (150mm to 300mm)	Regional Mixed Use Corridor	Yonge Street & Murihead Crescent	2041
W2.2	New 300 Watermain	Regional Mixed Use Corridor	Yonge Street between Murihead Crescent and Stouffville Road	2041
W2.3	New 200 mm Watermain	Regional Mixed Use Corridor	Yonge Street North of Murihead Crescent	2041
W3.1	New 250 mm Watermain	Regional Mixed Use Corridor	Yonge Street & Harris Avenue	2041
W3.2	New 300 Watermain	Regional Mixed Use Corridor	Yong Street between Harris Avenue and Jefferson Sideroad	2041
W3.3	New 300 Watermain	Regional Mixed Use Corridor	Yonge Street West Side, North of Gamble Road	2041
W4	New 250 mm Watermain	Regional Mixed Use Corridor	Yonge Street East Side, North of Silverwood Ave	2041
W5	New 300 mm Watermain	Key Development Area	South of Yonge St and Canyon Hill Ave	2041
W6.1	Watermain Replacement (150mm to 300mm)	Regional Mixed Use Corridor	Yonge St. North of Oxford St.	2051
W6.2	Watermain Replacement (150mm to 250mm)	Regional Mixed Use Corridor	Yong St. & Oxford St.	2051
W7.1	Watermain Replacement (150mm to 250mm)	Downtown Local Centre	Lorne Ave & Church St. North of Major Mackenzie	2041
W7.2	Watermain Replacement (150mm to 300mm)	Downtown Local Centre / Regional Mixed Use Corridor	Church St. South of Major Mackenzie	2051
W8.1	Watermain Replacement (150mm to 300mm)	Downtown Local Centre	Yonge St East Side between Major Mackenzie Dr. and Elmwood Ave.	2041

Project ID	Description	OP Area	Location	Year
W8.2	New 300 Watermain	Downtown Local Centre	Yonge St East Side between Major Mackenzie Dr. and Elmwood Ave.	2041
W8.3	New 150 mm Watermain	Downtown Local Centre	Yonge St & Palmer Ave.	2041
W9.1	Watermain Replacement (150mm to 250mm)	Loyal Development Area	Cedar Ave. South of Major Mackenzie Dr.	Ultimate
W9.2	Watermain Replacement (150mm to 250mm)	Loyal Development Area	Norfolk Ave. South of Major Mackenzie Dr.	2051
W10	Watermain Replacement (150mm to 300mm)	Loyal Development Area	Old Markham Rd. & Bayview Ave.	2041
W11.1	Watermain Replacement (150mm to 250mm)	Regional Mixed Use Corridor	Yonghurst Drive at Yonge Street Intersection	2041
W11.2	New 300 Watermain	Regional Mixed Use Corridor	Yonge Street East Side North of Clarissa Dr.	2041
W11.3	Watermain Replacement (150mm to 300mm)	Regional Mixed Use Corridor	Yonge Street East Side North of Clarissa Dr.	2041
W12.1	Watermain Replacement (150mm to 250mm)	Regional Mixed Use Corridor	Spruce Avenue At Yonge St. Intersection	2041
W12.2	Watermain Replacement (150mm to 250mm)	Regional Mixed Use Corridor	Oak Avenue at Yonge St. Intersection	2041
W12.3	Watermain Replacement (150mm to 250mm)	Regional Mixed Use Corridor	Edgar Avenue at Yonge St. Intersection	2041
W13.1	Watermain Replacement (150mm to 250mm)	Regional Mixed Use Corridor / Richmond Hill Centre	Scott Drive at Yonge St. Intersection	2041
W13.2	Watermain Replacement (150mm to 250mm)	Richmond Hill Centre	Mackay Drive at Yonge St. Intersection	Ultimate
W14.1	Watermain Replacement (150mm to 300mm)	Richmond Hill Centre	High Tech Road between Yonge St and Red Maple Rd.	2041
W14.2	New 300 Watermain	Richmond Hill Centre	High Tech Road between Yonge St and Red Maple Rd.	2041
W14.3	Watermain Replacement (150mm to 300mm)	Richmond Hill Centre	High Tech Road between Yonge St and Red Maple Rd.	2041
W15	Watermain Replacement (150mm to 250mm)	Richmond Hill Centre	Garden Avenue At Yonge St. Intersection	2041



Legend

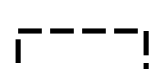







-  2014 UMESP Study Area
-  Emerging Growth Centres
-  Regional Watermain
-  Local Watermains
-  Proposed Watermain Replacement
-  Proposed New Watermain
-  Proposed District Boundary
-  City Boundary

Figure 6-2
Ultimate Water Solutions

Drawn By: W.A. Date: Aug 22, 2023



7.0 Implementation

7.1 Wastewater

The assessment of the interim scenarios (2041 and 2051), between the existing conditions and the Ultimate Build-out scenario, was used to establish the timing of recommended wastewater projects based on anticipated or estimated locations for development within the study area for each growth period. However, the actual triggers for individual projects would be development-driven, so the recommended timing of projects would need to be reviewed and adjusted periodically to account for the actual pace of growth. This is especially relevant to the preparation of applicable development charge bylaws. **Table 7-1** list the anticipated timing of the recommended wastewater projects.

Table 7-1: Anticipated Timing of Proposed Wastewater Solutions

Project ID	Implementation Phase		
	2041	2051	Ultimate Build out
WW-1	Partially	Partially	Fully
WW-2	Fully	Fully	Fully
WW-3	Partially	Partially	Fully
WW-4	Fully	Fully	Fully
WW-5	Fully	Fully	Fully
WW-6	Fully	Fully	Fully
WW-7	Fully	Fully	Fully
WW-8	Partially	Partially	Fully
WW-9	-	Partially	Fully
WW-10	Fully	Fully	Fully
WW-11	Partially	Partially	Fully
WW-12	Partially	Partially	Fully
WW-13	Partially	Partially	Fully
WW-14	Fully	Fully	Fully
WW-15	-	-	Fully
WW-16	-	-	Fully
WW-17	-	-	Fully
WW-18	Partially	Partially	Fully
WW-19	Fully	Fully	Fully
WW-20	-	-	Fully

7.1.1 Cost Estimates

AACE type Class 4 cost estimate has been employed to determine the preliminary cost estimates of the identified projects in the study area. Civica’s 2020 unit price database was used as a baseline for the cost estimate analysis. The unit prices were adjusted to 2023 prices using Infrastructure Cost Indexes from Statistics Canada. A trend analysis using data from 2010 to 2019 was used to extrapolate the index values to 2023. Moreover, a 25% multiplier for project delivery allowance and 25% for Class 4 Contingency have been applied to the cost estimates.

The project delivery allowance is a multiplier for preliminary design, tendering, construction services, insurance, mobilization & demobilization, traffic control, utility impacts/relocations, etc. The unit rates and costing estimates do not include any land costs.

The estimated cost of the recommended wastewater projects for the ultimate Build-out scenario is \$41.1M and includes HST.

Table 7-2 lists the total cost per recommended project, a detailed breakdown of the cost estimates is presented in **Appendix A**.

Table 7-2: Estimated Cost of Recommended Wastewater Projects

Project ID	2041 Scenario Total Cost	Additional Cost to Service 2051 Population	2051 Scenario Total Cost	Additional Cost to Service Ultimate Build out Population	Ultimate Build out Total Cost
WW-1	\$855,092	\$0	\$855,092	\$810,031	\$1,665,123
WW-2	\$2,444,174	\$0	\$2,444,174	\$0	\$2,444,174
WW-3	\$581,355	\$0	\$581,355	\$1,145,080	\$1,726,435
WW-4	\$381,896	\$0	\$381,896	\$0	\$381,896
WW-5	\$205,766	\$0	\$205,766	\$0	\$205,766
WW-6	\$401,508	\$0	\$401,508	\$0	\$401,508
WW-7	\$3,005,095	\$0	\$3,005,095	\$0	\$3,005,095
WW-8	\$1,458,088	\$0	\$1,458,088	\$554,944	\$2,013,032
WW-9	\$0	\$595,442	\$595,442	\$310,133	\$905,575
WW-10	\$1,276,321	\$0	\$1,276,321	\$0	\$1,276,321
WW-11	\$1,357,603	\$238,247	\$1,595,850	\$1,393,934	\$2,989,784
WW-12	\$1,083,446	\$0	\$1,083,446	\$1,679,548	\$2,762,994
WW-13	\$2,506,646	\$288,988	\$2,795,634	\$3,742,290	\$6,537,924
WW-14	\$1,800,114	\$0	\$1,800,114	\$0	\$1,800,114
WW-15	\$0	\$0	\$0	\$841,679	\$841,679
WW-16	\$0	\$0	\$0	\$1,164,700	\$1,164,700
WW-17	\$0	\$0	\$0	\$3,320,329	\$3,320,329
WW-18	\$148,879	\$0	\$148,879	\$158,796	\$307,675
WW-19	\$0	\$0	\$0	\$5,997,674	\$5,997,674
WW-20	\$0	\$0	\$0	\$1,315,721	\$1,315,721
Total	\$17,505,983	\$1,122,677	\$18,628,660	\$22,434,859	\$41,063,519

7.2 Water

The deficient fire flows and high pressures occur when the indicated population growth happens, which is under 2041 conditions for most OP intensification areas and all development applications. Therefore, all but two (2) of the recommended improvement projects will be required prior to 2041, either fully or partially.

Indeed, only projects W-6 and W-9 are recommended for construction by 2051 when the population growth is forecasted to start. **Table 7-3** summarizes the implementation phases for each project.

Table 7-3. Anticipated Timing of Proposed Water Projects

Project ID	Implementation Phase		
	2041	2051	Ultimate Build out
W-1	Fully	-	-
W-2	Fully (W2.1, W2.2 & W2.3)	-	-
W-3	Fully (W3.1, W3.2 & W3.3)	-	-
W-4	Fully	-	-
W-5	Fully (W5.1 & W5.2)	-	-
W-6	-	Fully (W6.1 & W6.2)	-
W-7	Partially (W7.1)	Partially (W7.2)	-
W-8	Fully (W8.1, W8.2 & W8.3)	-	-
W-9	-	Partially (W9.2)	Partially (W9.1)
W-10	Fully	-	-
W-11	Fully (W11.1, W11.2 & W11.3)	-	-
W-12	Fully (W12.1, W12.2 & W12.3)	-	-
W-13	Partially (W13.1)	-	Partially (W13.2)
W-14	Fully (W14.1, W14.2 & W14.3)	-	-
W-15	Fully	-	-

7.2.1 Cost Estimate

AACE type Class 4 cost estimate has been employed to determine the preliminary cost estimates of the identified projects in the study area. Civica’s 2020 unit price database was used as a baseline for the cost estimate analysis. The unit prices were adjusted to 2023 prices using Infrastructure Cost Indexes from Statistics Canada. A trend analysis using data from 2010 to 2019 was used to extrapolate the index values

to 2023. Moreover, a 25% multiplier for project delivery allowance and 25% for Class 4 Contingency have been applied to the cost estimates. The project delivery allowance is a multiplier for preliminary design, tendering, construction services, insurance, mobilization & demobilization, traffic control, utility impacts/relocations, etc.

The total cost of the proposed projects is \$ **21,668,980** and includes HST. More than 70% of the cost is proposed by 2041. **Table 7-4** summarizes the estimated cost per project and per implementation phase. Details are presented in **Appendix B**.

Table 7-4. Proposed Water System Projects – Cost Estimate

Project	2041(\$)	2051(\$)	Build Out(\$)	Total Cost (2023)(\$)
W1	400,209	-	-	400,209
W2.1	293,996	-	-	293,996
W2.2	949,595	-	-	949,595
W2.3	179,444	-	-	179,444
W2 Subtotal	1,423,035	-	-	1,423,035
W3.1	226,330	-	-	226,330
W3.2	960,621	-	-	960,621
W3.3	547,119	-	-	547,119
W3 Subtotal	1,734,071	-	-	1,734,071
W4	1,666,022	-	-	1,666,022
W5	1,417,573	-	-	1,417,573
W6.1	-	163,898	-	163,898
W6.2	-	218,101	-	218,101
W6 Subtotal	-	381,999	-	381,999
W7.1	1,855,100	-	-	1,855,100
W7.2	-	3,606,203	-	3,606,203
W7 Subtotal	1,855,100	3,606,203	-	5,461,303
W8.1	848,983	-	-	848,983
W8.2	457,982	-	-	457,982
W8.3	147,101	-	-	147,101
W8 Subtotal	1,454,067	-	-	1,454,067
W9.1	-	-	550,041	550,041
W9.2	-	522,027	-	522,027
W9 Subtotal	-	522,027	550,041	1,072,068
W10	937,539	-	-	937,539
W11.1	81,305	-	-	81,305
W11.2	546,810	-	-	546,810
W11.3	346,433	-	-	346,433
W11 Subtotal	974,547	-	-	974,547
W12.1	498,929	-	-	498,929
W12.2	439,537	-	-	439,537
W12.3	403,367	-	-	403,367
W12 Subtotal	1,341,833	-	-	1,341,833
W13.1	485,805	-	-	485,805

Project	2041	2051	Build Out	Total Cost (2023)
W13.2	\$ -	\$ -	\$ 578,173	\$ 578,173
W13 Subtotal	\$ 485,805	\$ -	\$ 578,173	\$ 1,063,979
W14.1	\$ 366,785	\$ -	\$ -	\$ 366,785
W14.2	\$ 847,386	\$ -	\$ -	\$ 847,386
W14.3	\$ 627,636	\$ -	\$ -	\$ 627,636
W14 Subtotal	\$ 1,841,807	\$ -	\$ -	\$ 1,841,807
W15	\$ 498,929	\$ -	\$ -	\$ 498,929
Total	\$ 16,030,537	\$ 4,510,228	\$ 1,128,214	\$ 21,668,980

8.0 Public Consultation

As described in **Section 1.0**, this UMESP has been undertaken to address the requirements of a Class Environmental Assessment Master Plan, requiring Phase 1 and Phase 2. One requirement of Phase 2 is the need to consult with review agencies and the public once alternative solutions have been identified. Typically, consultation involves presenting the problem or opportunity that will be addressed, the environmental considerations and potential impacts of each alternative, and the approach used for evaluating the alternatives. The comments and the input from the public and other stakeholders are taken into consideration in the identification of the preferred alternative.

Consultation early and throughout the process is a key feature of environmental assessment planning. The purpose of the consultation process is to notify stakeholders of the project details and provide an opportunity for interested parties to review and submit comments related to the study. The following public and stakeholder consultation activities were completed throughout the Master Planning process.

8.1 Public Consultation

In accordance with the Class EA Master Plan framework, public consultation undertaken in association with this project included the following:

- Issuance of Notice of Commencement August 30, 2018
- Public Information Centre July 20, 2023

Public consultation materials, including the above notices, attendance information, and display boards presented at the public information centres, are provided in **Appendix C**.

9.0 References and Resources

1. City of Richmond Hill. May 2014. *“Urban Master Environmental Servicing Plan”*.
2. Municipal Engineers Association. October 2000 (as amended in 2007 and 2011). *“Municipal Class Environmental Assessment”*.
3. York Region. June 2023. *“Official Plan. Office Consolidation”*.
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6. City of Richmond Hill. November 2021. *“Key Directions Report – For the Richmond Hill Official Plan Update”*.
7. Town of Newmarket. September 2018. *“Design Standards, Town of Newmarket”*.
8. City of Toronto. January 2021. *“Design Criteria for Sewers and Watermains”*.
9. City of Waterloo. March 2015. *“Water Distribution Master Plan”*.
10. City of Burnaby. November 2019. *“Design Criteria Manual, Engineering Department”*.
11. City of Winnipeg. November 2019. *“Water Demand Estimation and Design Guidelines”*.
12. Town of Whitchurch-Stouffville. November 2017. *“Water and Wastewater Master Plan Study”*.
13. City of Edmonton. April 2021. *“Design and Construction Standards, Volume 4”*.
14. Fire Underwriters Survey. 2020 *“Water Supply for Public Fire Protection”*.